

Final MBR-Network Workshop

**“Salient outcomes of the European R&D
projects on MBR technology”**

Presentation handouts

31 March – 1 April, Berlin 2009 (Germany)



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30. THE EFFECT OF ORGANIC PEAK LOADS ON MEMBRANE BIOFOULING IN A PILOT SCALE MBR TREATING MUNICIPAL WASTEWATER

*Ben Zwickenpflug, Marc Boehler, Adriano Joss,
Hansruedi Siegrist*

The effect of organic peak loads on membrane biofouling in a pilot scale MBR treating municipal wastewater

Ben Zwickenpflug, Marc Boehler,
Adriano Joss, Hansruedi Siegrist



Content

Content

- ▶ Objectives
- ▶ *Euombra*-Pilot Plant
- ▶ Peak load experiments
- ▶ Results
- ▶ Conclusions

Objectives

Content

Objectives

For testing the interrelationship between peak loads and permeability, different kinds of organic substrates have been dosed, to provoke membrane fouling

- ✓ pilot scale MBR plant with state of the art modules
- ✓ municipal wastewater
- ✓ flow proportional influent
- ✓ operating conditions according to manufacturer's specifications

Eurombra Pilot Plant

Content

Objectives

Eurombra-Plant

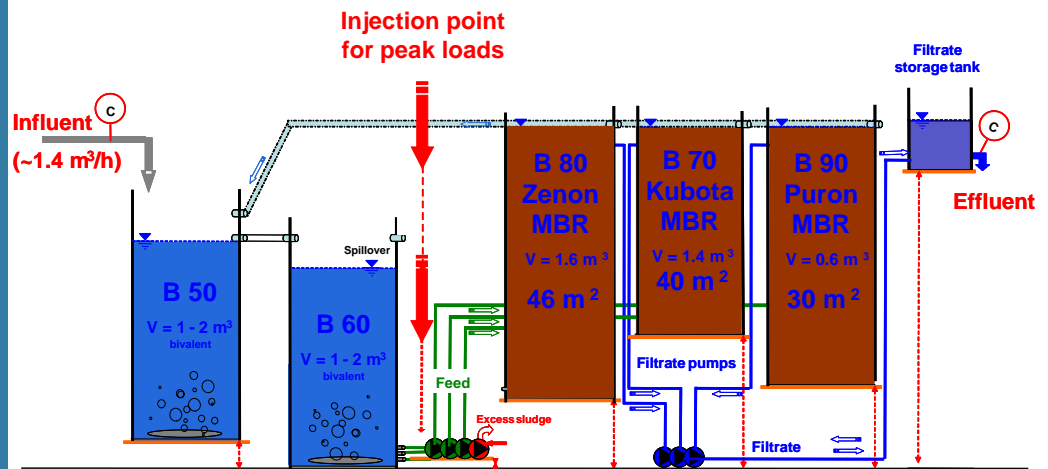


Eurombra Pilot Plant

Content

Objectives

Eurombra-Plant

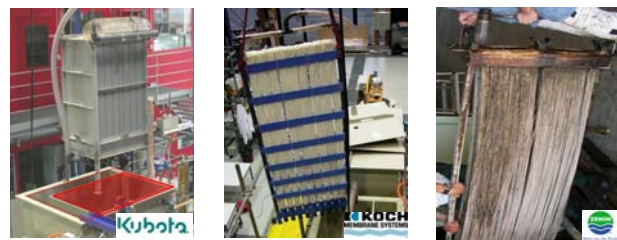


Installed full scale modules

Content

Objectives

Eurombra-Plant



	Kubota FS50	Puron A-30-HS	Zenon ZW500A
Geometry (mm between plates or diameter)	Flat sheet (7)	Hollow fibre (2)	Hollow fibre (2)
Pore size [µm]	0.4	0.1	0.04
Membrane Area [m²]	40	30	46
Maximal gross flux [m³·h⁻¹]	1.5	1.5	1.5
Aeration rate [Nm³·h⁻¹]	50	15	45
Air cycling (On / Off) [s]	No	No	10 / 10
Permeate cycle: production/pause (backpulse) [s]	480 / 120 (0)	480 / 120 (30) or 300 / 60 (25)	480 / 120 (25)
Chamber volume [m³]	1.41	0.67	1.61
Chamber size (LxWxH) [m]	1.1 x 0.8 x 1.60	1.2 x 0.25 x 2.25	Ø 1.0 x 2.05

Experiments

Content

Objectives

Eurombra-Plant

Experiments

Dry weather conditions:

COD load by wastewater influent = 13.4 kg COD_{tot} / d

COD_{sol} / COD_{part} = 1

▶ Experiment 1 and 2:

Doubling the COD_{sol} load by a daily dosing of 6 kg COD of
- sodium acetate
- or sugar

▶ Experiment 3:

Doubling the COD_{part} load by a daily dosing of 6 kg COD of
- wheat flour

- ▶ Duration of each experiment: **7 days** in a row, with chemical cleaning and a regeneration period of 2 weeks in between the experiments
- ▶ Injection of peak load solution: daily at 10:00
- ▶ Duration of dosage: **7- 12 minutes**

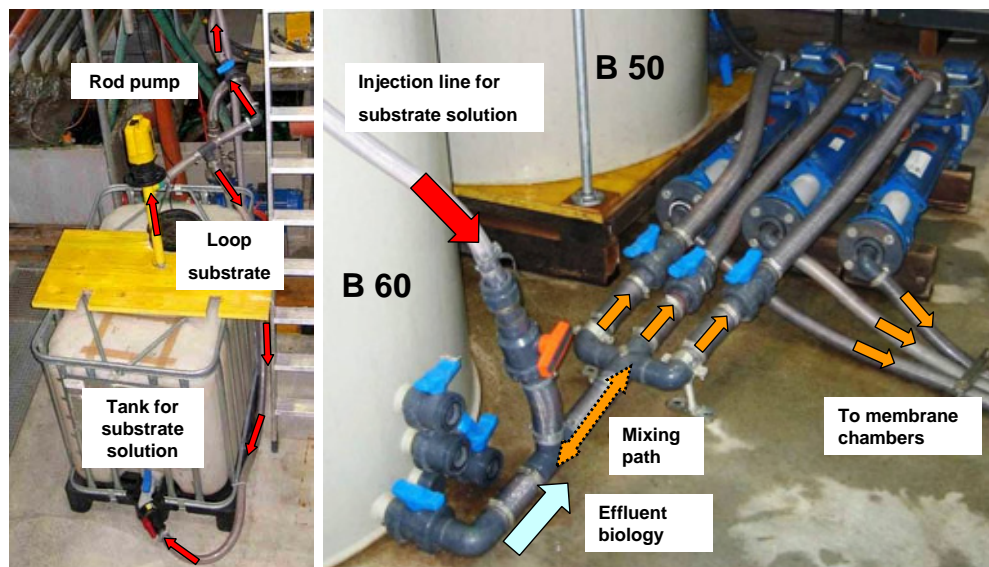
Set up

Content

Objectives

Eurombra-Plant

Experiments



Problems: Loss of TSS due to big foam events

Content

Objectives

Eurombra-Plant

Experiments



COD_{sol} in sludge filtrate (paper filtered) and effluent (membrane filtered)

Content

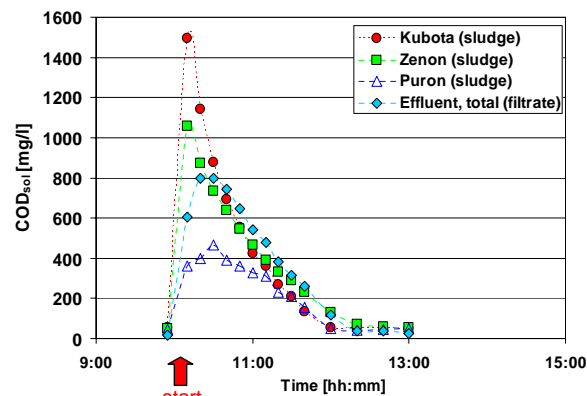
Objectives

Eurombra-Plant

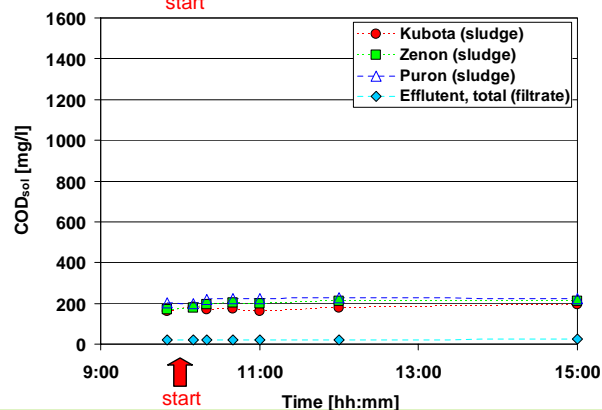
Experiments

Results

Acetate Dosage
02.12.2009 day
725

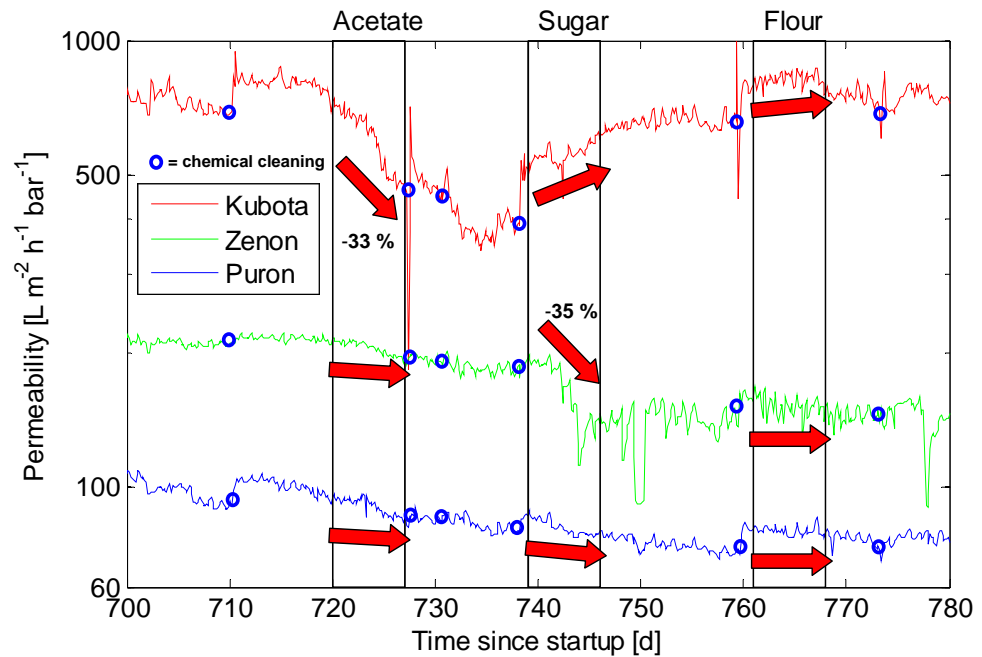


Flour Dosage
10.01.2009 day
765



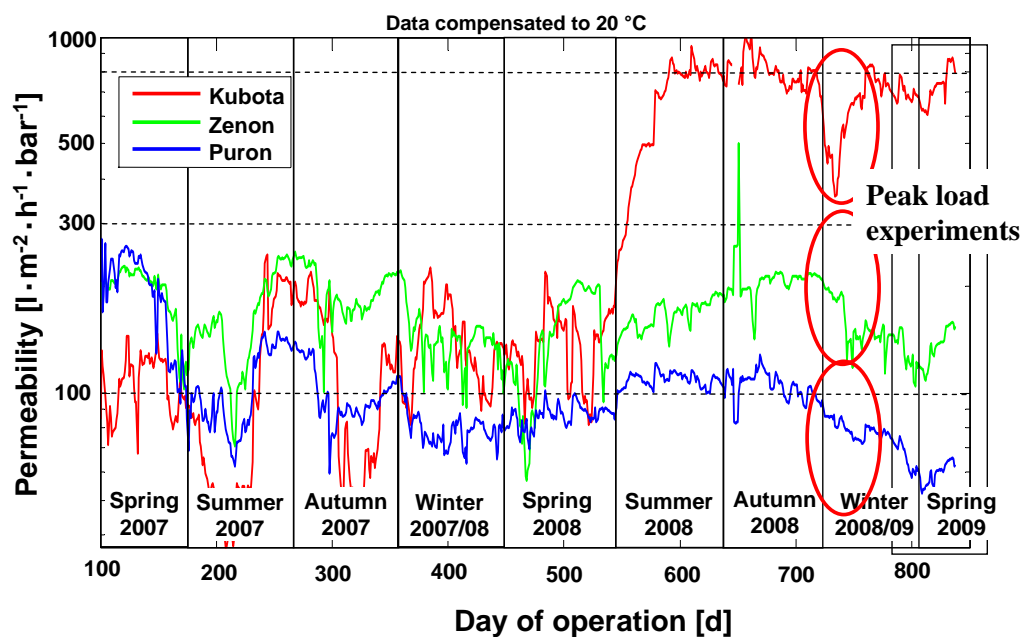
Impact of peakload experiments on membrane permeability

- Content
- Objectives
- Eurombra-Plant
- Experiments
- Results



Long-term Permeability

- Content
- Objectives
- Eurombra-Plant
- Experiments
- Results



Spring: 01.03. – 31.05. Summer: 01.06. – 31.08.
 Autumn: 01.09. – 30.11. Winter: 01.12. – 28.02.

Conclusions

Content

Objectives

Eurombra-Plant

Experiments

Results

Conclusions

- ▶ High organic loading rates causing high F/M ratios seem to have only minor influence on membrane permeability compared to other unidentified factors, e.g. sludge characteristics or seasonal variations
- ▶ Literature findings that predicted extremely high fouling rates (decrease of permeability) could not be confirmed in our experiments
- ▶ Fouling caused by high F/M ratios seems to be mostly reversible
- ▶ Decline of permeability has been observed only for dosage of COD_{sol} not for COD_{part}

Acknowledgement

EUROMBRA is a research project supported by the European Commission under the Sixth Framework Programme (Priority “Global Change and Ecosystems”)



Contract No. 018480 - EUROMBRA

Duration: 01/10/05 – 31/05/09

EUROMBRA is part of the MBR-NETWORK Cluster



More info: www.mbr-network.eu

31. DECISION TREE FOR FULL-SCALE SUBMERGED MBR CONFIGURATIONS

H. De Wever, C. Brepols, B. Lesjean

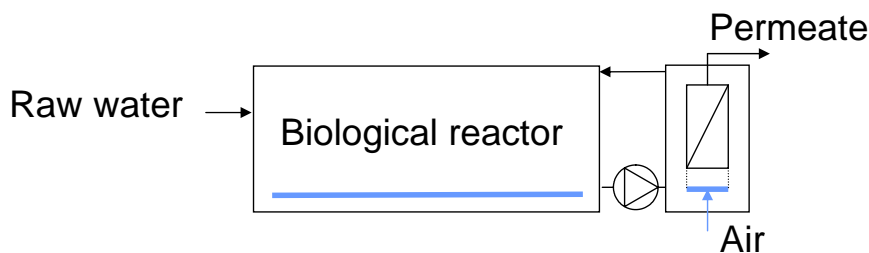
Decision tree for full-scale submerged MBR configurations

H. De Wever, VITO
C. Brepols, Erftverband
B. Lesjean, KWB

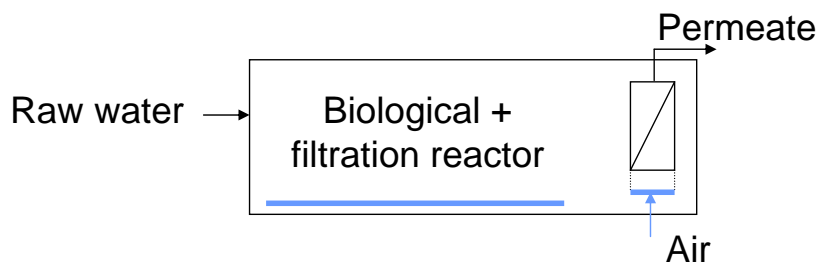


Submerged MBR configurations

▶ Separate system



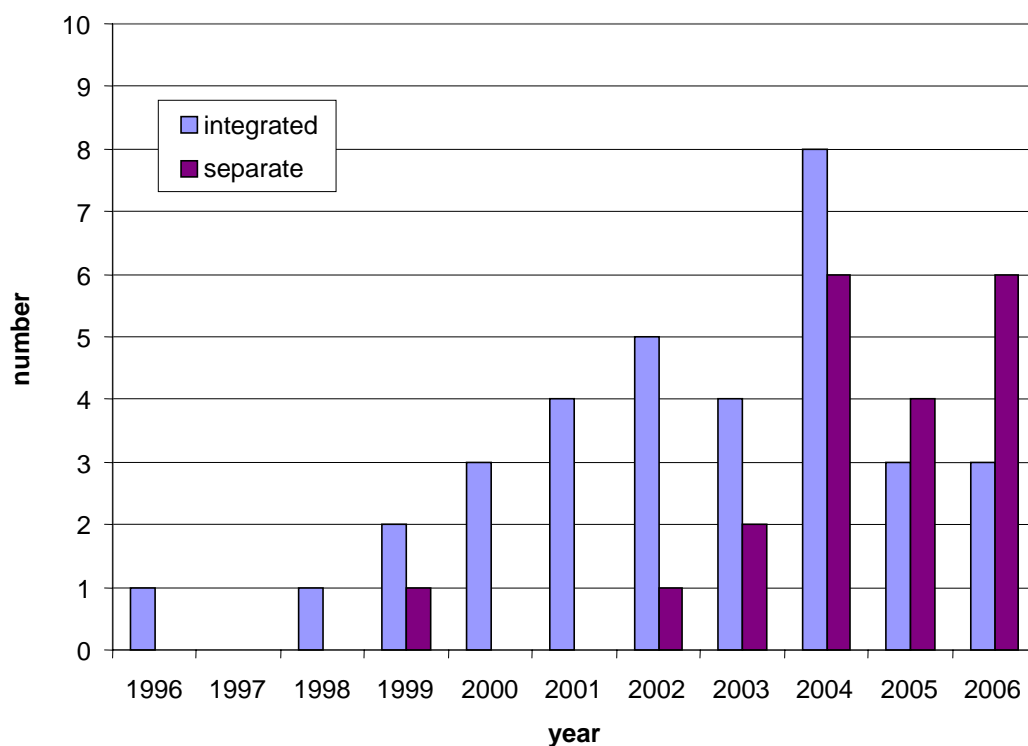
▶ Integrated system



Approach for comparison configurations

- ▶ Trends for full-scale municipal MBRs in Europe
- ▶ Comparative pilot trials
- ▶ Questionnaire for selected operators/end-users or suppliers
- ▶ Literature review
- ▶ Comparison of 2 full-scale MBRs
- ▶ Conclusions: Decision tree

Trends for full-scale municipal MBRs in Europe



Trends for full-scale municipal MBRs in Europe

- ▶ 98 commissioned by 2006 – sufficient details found for 54
 - 34 integrated submerged (total capacity 6018 m³/h)
 - 20 separate submerged (total capacity 6744 m³/h)

	C removal	N removal		Retrofit	New
Integrated	19	15	Integrated	19	14
Separate	3	13	Separate	10	10

	Flat sheet	Hollow fibre
Integrated	23	11
Separate	6	14

- Larger plants > 200 m³/h: preference for separate configuration

Literature review

- ▶ Tendency towards separate configuration
 - Advantages
 - Easy access for maintenance, cleaning
 - Lower fouling – better control hydraulics, biological system
 - Different sludge distribution
 - Cascading effect: less susceptible to fluctuations in flow or load
 - Better effluent quality – advanced nutrient removal
 - Higher operational flexibility
 - Disadvantages
 - Larger footprint
 - Higher investment and operational costs

Comparison of 2 full-scale MBRs

- ▶ Kaarst-Nordkanal (integrated) <> Varsseveld (separate)
 - Both equipped with Zenon membranes
 - Operational and cost data available
 - Comparable designs
 - Nordkanal: capacity 4x higher than for Varsseveld
 - Comparable discharge consents except for total N

Sources: van Bentem et al. (2007), Brepols (2008)

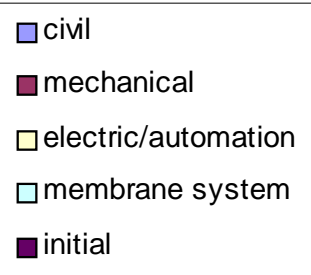
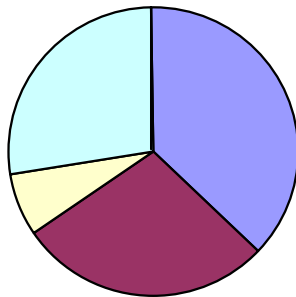
Comparison of 2 full-scale MBRs

Parameter	Kaarst-Nordkanal	Varsseveld
Treated flow per year (m ³)	5 236 000	1 825 000
Sludge load (kg BOD/kg MLSS.d)	< 0.05	0.04
Sludge retention time (d)	25	24-35
Aeration	Intermittent (15 s off/15 s on)	Intermittent (15 s off/15 s on)
Specific aeration demand (Nm ³ air/h.m ²)	0.43	0.55
Specific aeration demand (Nm ³ air/m ³ permeate)	17	15
Physical cleaning	every 800 s backflush for 60 s	every 360 s backflush for 20 s
Chemical cleaning	maintenance clean every 1 or 2 weeks	maintenance clean every 1 or 2 weeks

Comparison of 2 full-scale MBRs

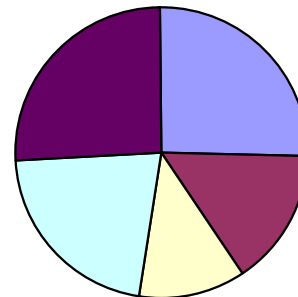
Capital costs

Kaarst



270 EUR/PE
1 500 EUR/m³/d
(integrated)

Varsseveld

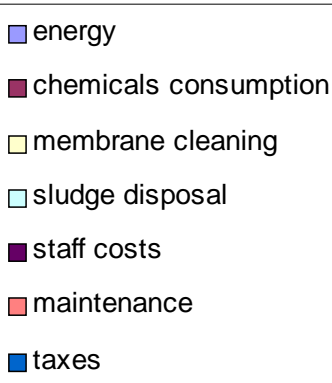
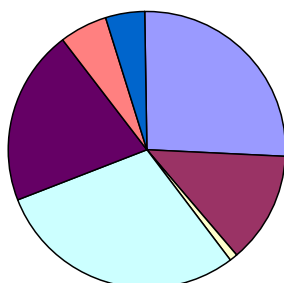


490 EUR/PE
2 250 EUR/m³/d
(separate)

Comparison of 2 full-scale MBRs

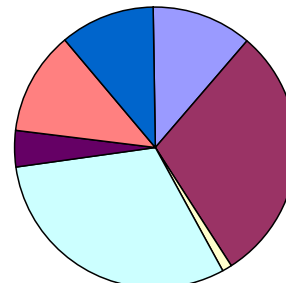
Operational costs

Kaarst



17 EUR/PE
0.26 EUR/m³ treated
(integrated)

Varsseveld

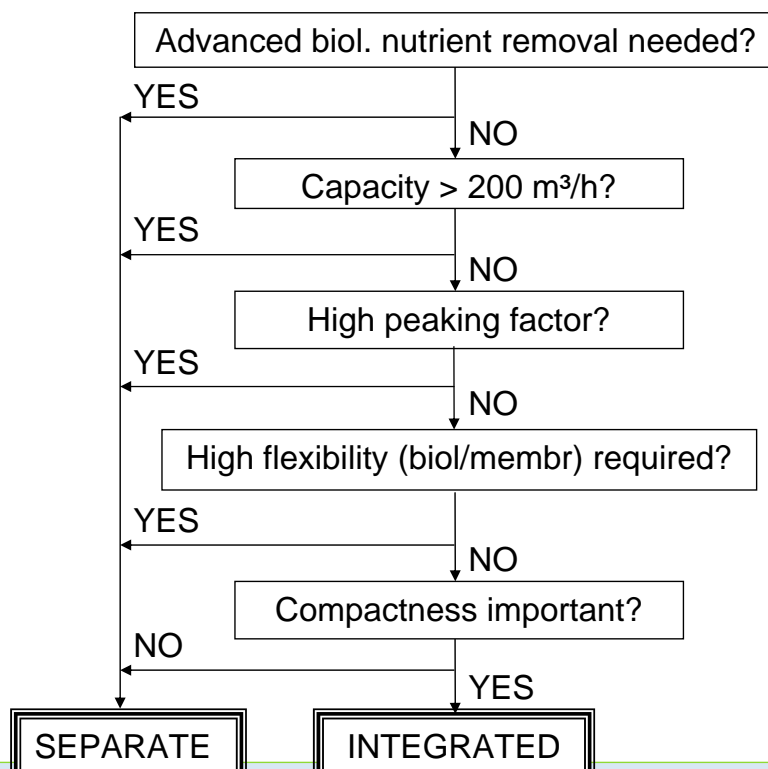


29 EUR/PE
0.36 EUR/m³ treated
(separate)

Comparison of 2 full-scale MBRs

Energy consumption (kWh/m ³)	Kaarst-Nordkanal	Varsseveld
Membrane aeration	0.23	0.34
Bioreactor aeration	0.30	0.24
<i>Total aeration</i>	<i>0.53</i>	<i>0.58</i>
Membrane supply pumps	0.03	0.11
Permeate pumps	0.04	0.12
Bioreactor impellers	0.05	0.04
Others	0.25 (incl. sludge dewatering)	0.05
<i>Total specific power demand</i>	<i>0.9</i>	<i>0.9</i>

Conclusions: Decision tree



Acknowledgement

AMEDEUS is a research project supported by the European Commission under the Sixth Framework Programme (Priority "Global Change and Ecosystems")



Contract No. 018328 - AMEDEUS
Duration: 01/10/05 – 31/05/09
AMEDEUS is part of the MBR-NETWORK Cluster



More info: www.mbr-network.eu

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**32. OPERATIONAL EXPERIENCE OF MBR
DEMONSTRATION PLANT WITH POST
DENITRIFICATION IN BERLIN-
MARGARETENHÖHE (ENREM-PROJECT)**
C. Lüdicke, J. Stüber, R. Gnirss, B. Lesjean, M.
Kraume

MBR Demonstration Plant: EBPR with Post Denitrification in Berlin- Margaretenhöhe

Regina Gnirss, Carsten Lüdicke, Berliner Wasserbetriebe
Boris Lesjean, Johan Stüber, KompetenzZentrum Wasser Berlin
Mathias Kraume, IVT TU Berlin



Structure



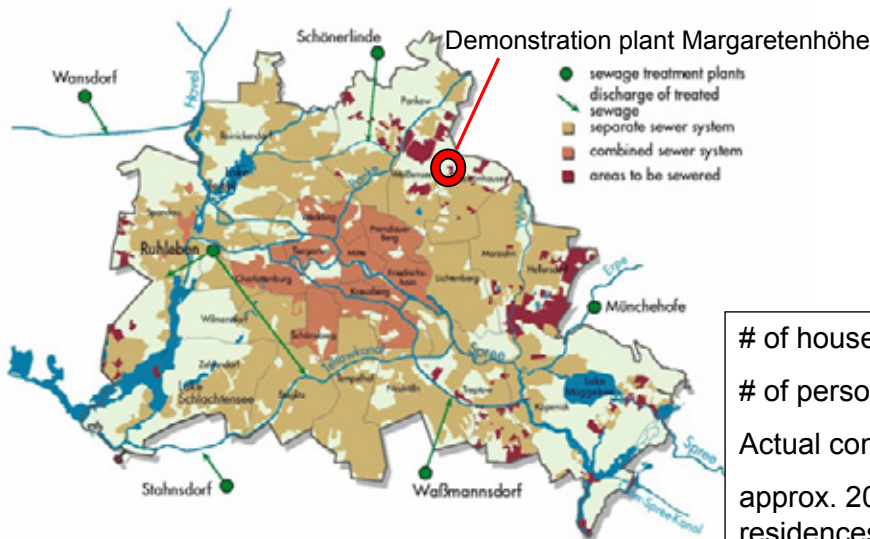
- Motivation
- Aim
- Process
- Results
- Energy consumption
- Conclusions

- Primary sedimentation
- Conventional Aeration Process:
 - Carbon: COD < 50 mg/L 96%
 - EBPR: TP < 0,5 mg/L 96%
 - Nitrogen: TN < 18 mg/L 80%
- Sedimentation
- Sludge Treatment: Digestion or Incineration

} 6 Large WWTP



- ➔ About 33,000 inhabitants of Berlin have no sewer connection (1%)
- ➔ Only drainoff free septic tanks allowed
 - ➔ expensive wastewater disposal
 - ➔ risks of leaks and pollution



of households: ~ 90
 # of persons: ~ 230
 Actual connection rate: ~ 100 %
 approx. 20 % summer residences



Enhanced Nutrients Removal in Membrane Bioreactor (Vermehrte Nährstoffelimination in Membranbelebungsreaktoren)

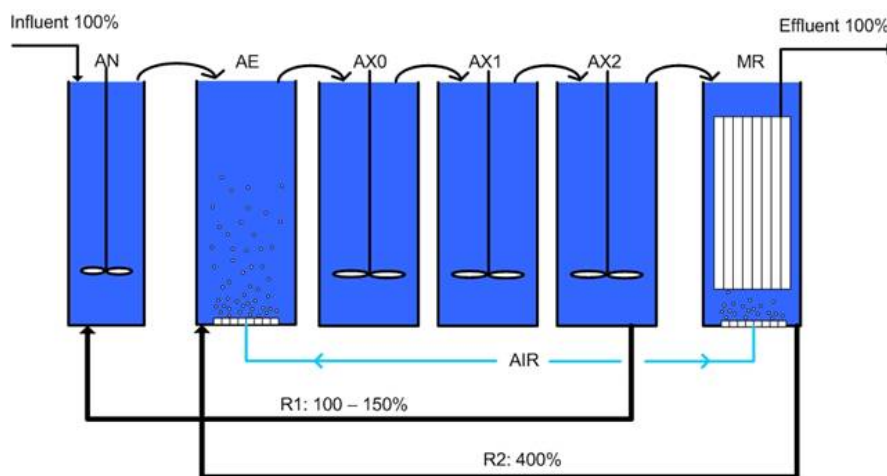
- Demonstration of a novel process for enhanced biological nutrients removal
- Achievement of better effluent quality than the large WWTP in Berlin
- Evaluation of the operational costs and energy consumption

- Process was developed in a 3-year research project IMF
- Patent-protected by Berliner Wasserbetriebe und Veolia Wasser



ENREM process

Combining of biological phosphorus removal and post - denitrification



Goal:
 COD < 50 mg/l
 TN < 10 mgN/l
 TP < 0,1 mgP/l
 Desinfection

Feb. 2008 Conversion AE2 – AX0



ENREM process

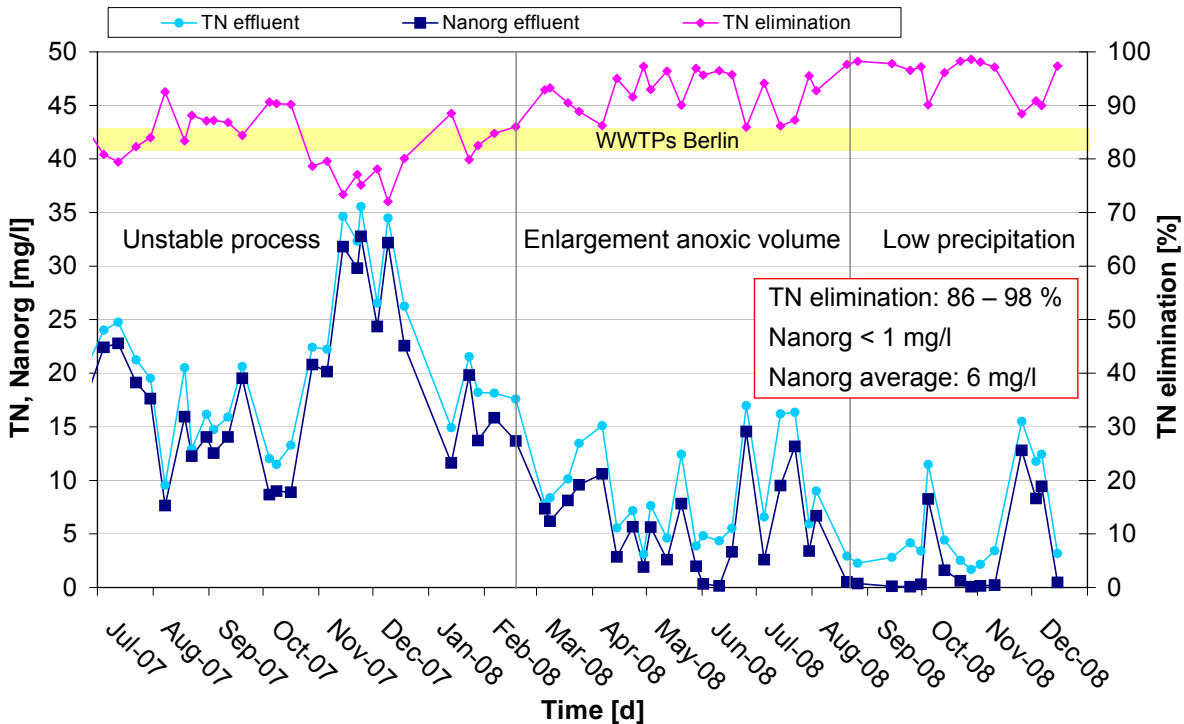


- + Containerised plant
- + Sieving within the container
- + Maintenance entry via top
- + Plant investment costs

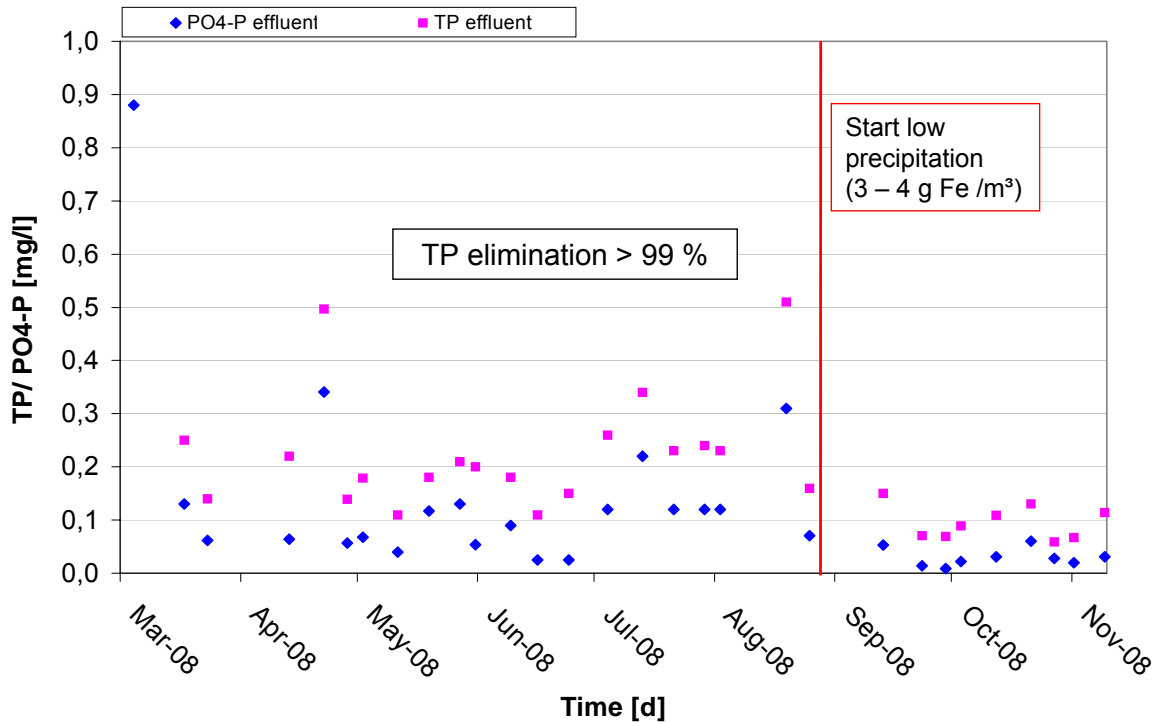
260.000 €



Nitrogen elimination (24h sample)



Phosphorus elimination (24h sample)



Membrane cleaning



Cleaning Objectives

- Preventive cleaning
- Low chemical concentration
- Low cleaning efforts

Module 4 (chemical soaking)	Module 5 (chemical soaking)
H ₂ O ₂	Active Chlorine
1000 ppm / pH11	500 ppm
4 weeks	4 weeks

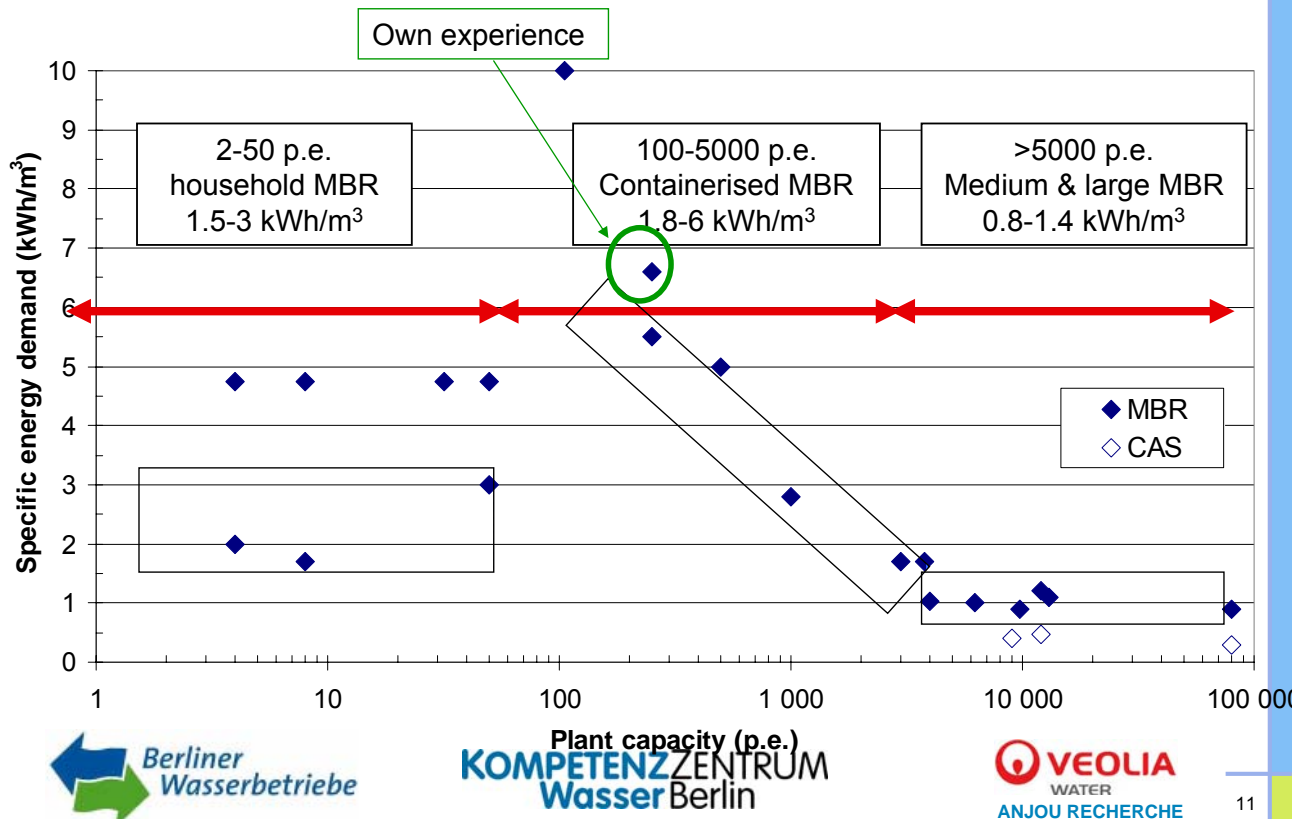
Cleaning strategy

- Alternating module operation
- 1 month operation / 1 month storage in chemical solution
- Cleaning with citrid acid when necessary (approx. every 3rd cleaning)

→ Cleaning results satisfying:

Permeability: 800 to 1200 L / (m² * h * bar)





Conclusions

- ➔ Plant is running for around 3 years without heavy breakdowns
- ➔ Under design load conditions the biological ENREM process works reliably and TP between 0.05 - 1 mg/L in grab sample
- ➔ Zone conversion (AE -> AX) and minimum precipitation stabilised the process
- ➔ Over 99% P-elimination (0.05 mg/L), over 95% N-elimination (< 6 mg/L)
- ➔ Monthly maintenance membrane cleaning caused high reliability of filtration with H₂O₂ or chlorine detergents



Thank you for your attention



Acknowledgment:

This study was conducted in the frame of the demonstration project ENREM, with subventions of the European LIFE-program (LIFE 04 ENV/DE/058) and financial support from Veolia Water and Berliner Wasserbetriebe.



➔ Backup





of households: ~ 90
 # of persons: ~ 230
 Actual connection rate: ~ 100 %
 approx. 20 % summer residences

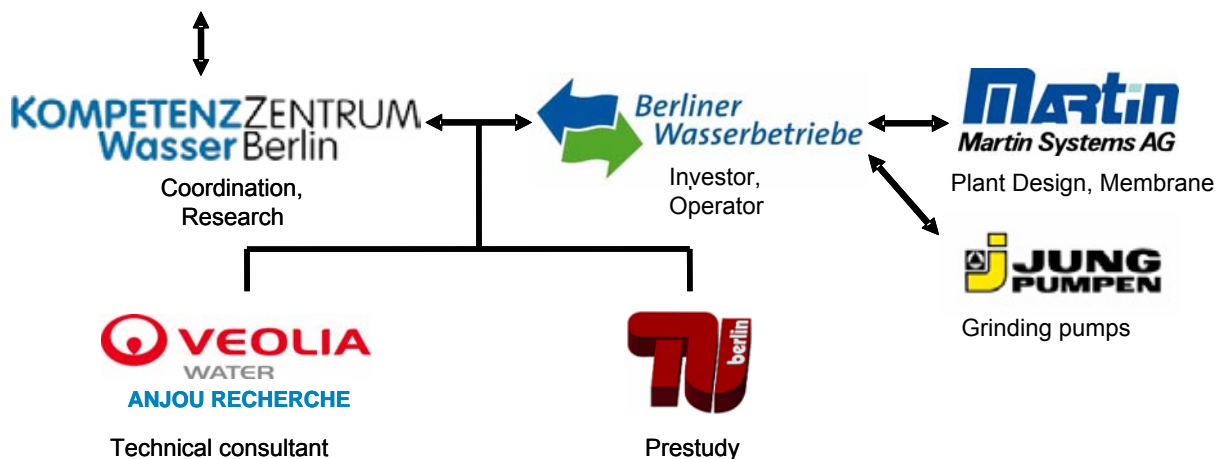


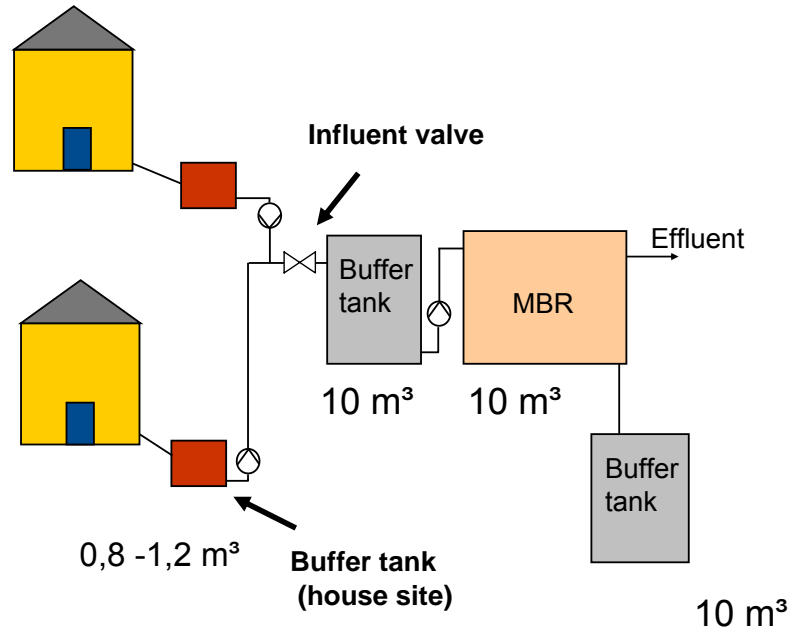
Project Structure

→ Demonstration project:



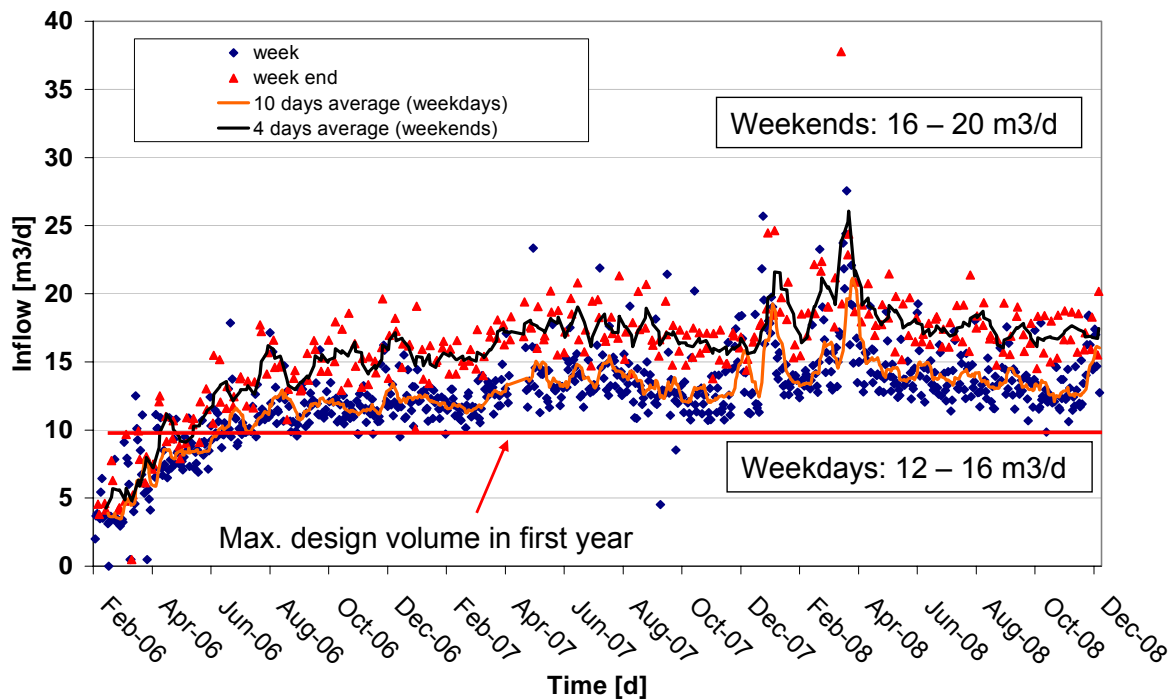
- Duration: 2004-2007 (3.5 years)
- Budget: ca. 2.4 Mio.€ (≈ 250k€ LIFE-subventions)





Buffer tank: Homogenization of waste water inflow and concentrations

→ High reliability in running the plant

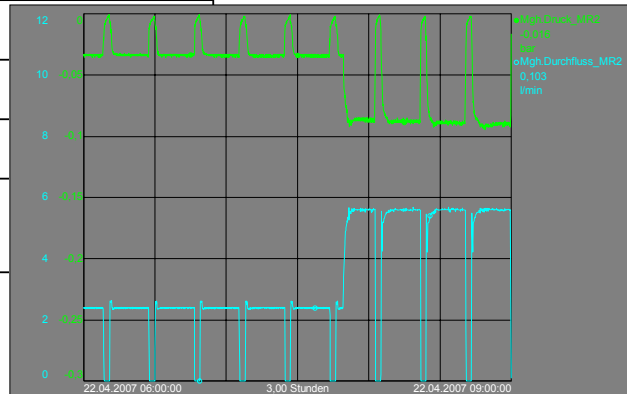


→ Extreme inflow variation (Storm water events in spring 2008)

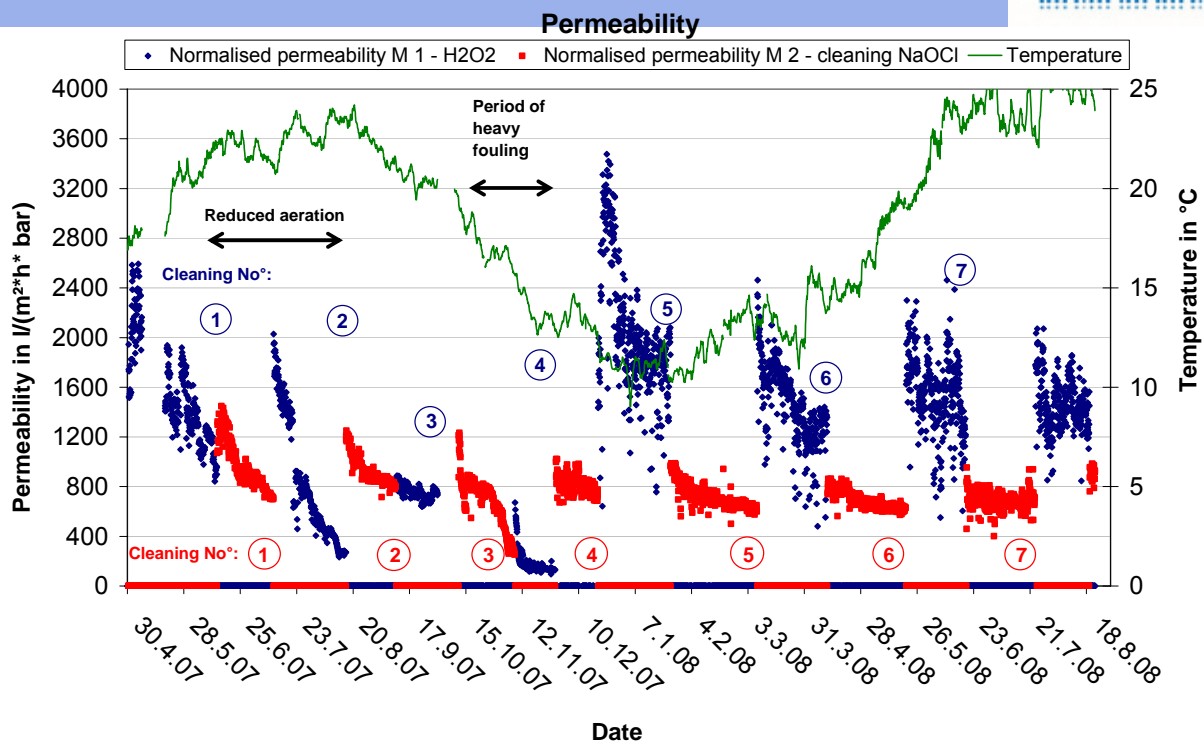
Filtration parameter



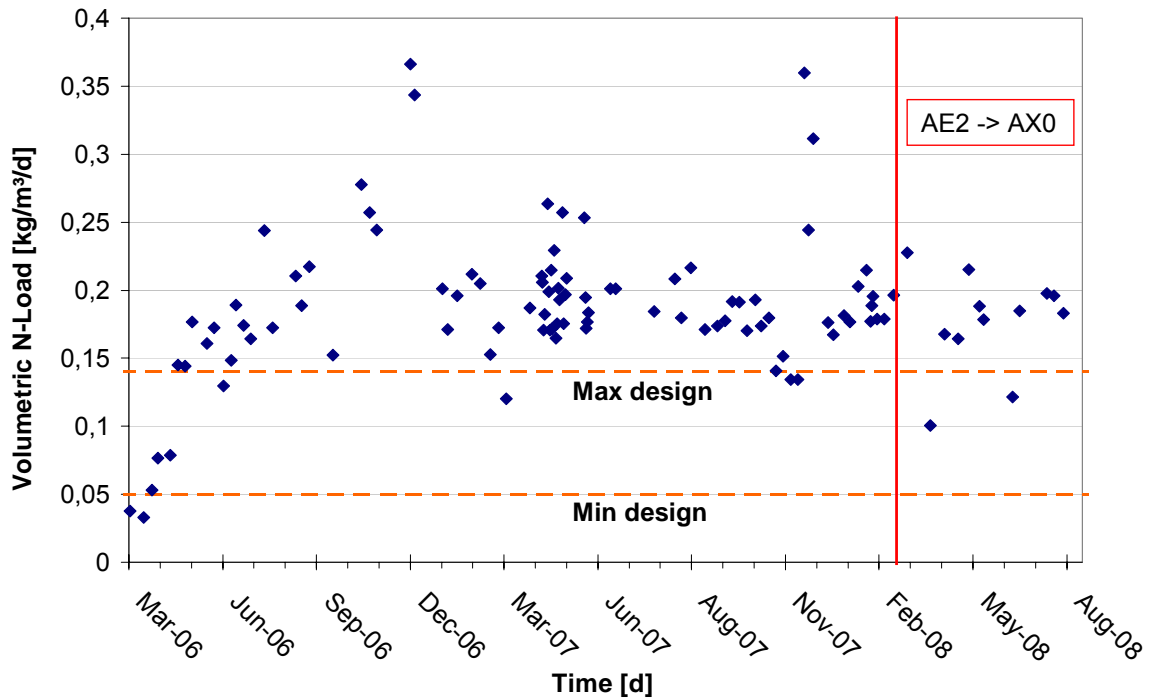
Type	Flat sheet
Material	Polyvinylidene fluoride (PVDF)
Pore diameter	200 nm (microfiltration)
Membrane area	31.8 m ² (per line / double deck)
TMP max.	300 mbar
TMP operation	20 – 30 mbar
Operating flux	15 – 20 L/m ² /h
Filtration time	999 sec
Relaxation time	143 sec No backwash



Membrane performance



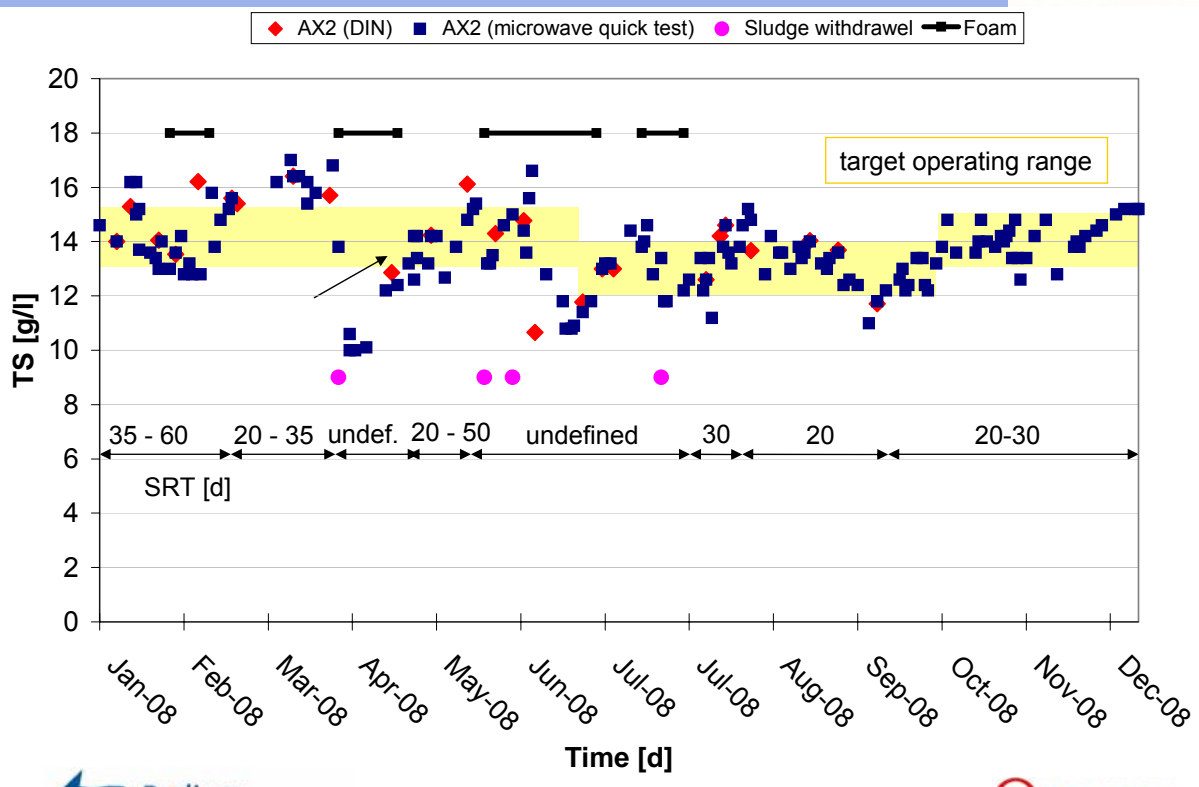
Volumetric nitrogen load



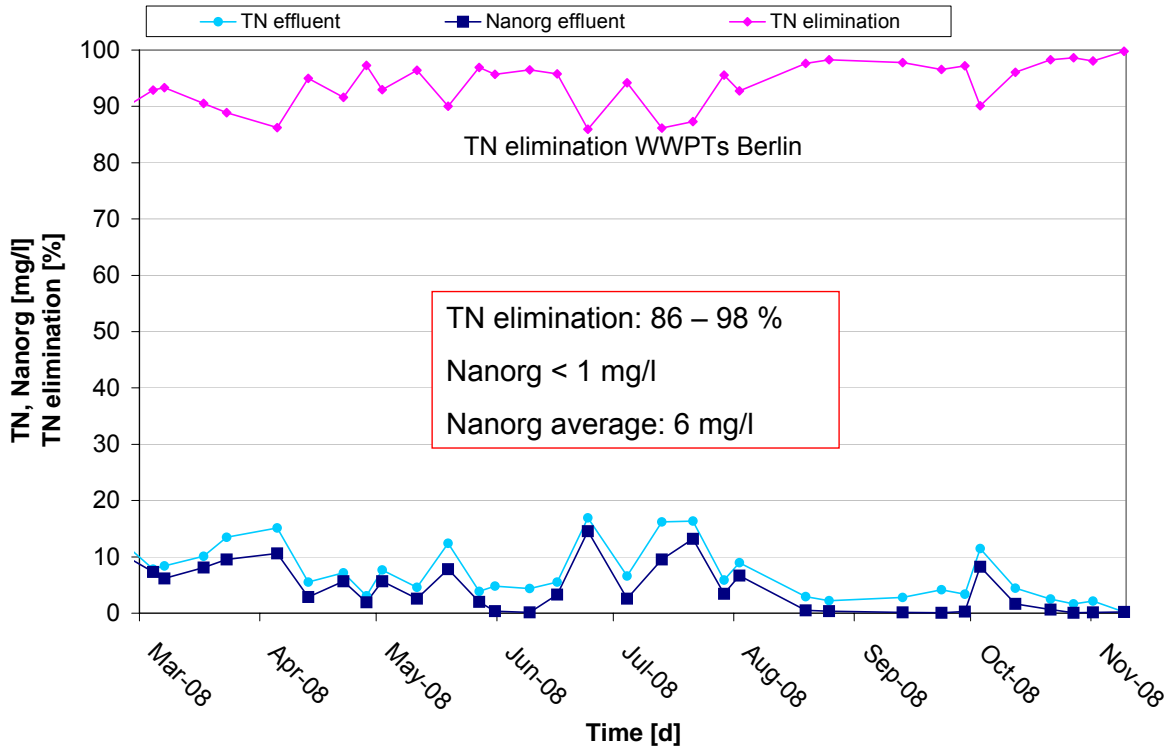
→ 20 – 50 % overload (same for phosphorus)



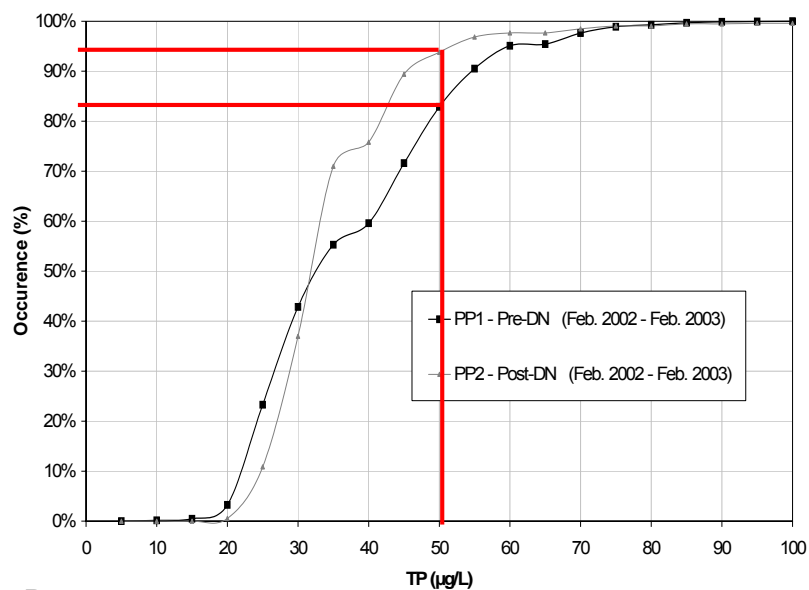
TS concentration



Nitrogen elimination (24h grab sample)



Phosphorus removal: EBPR only



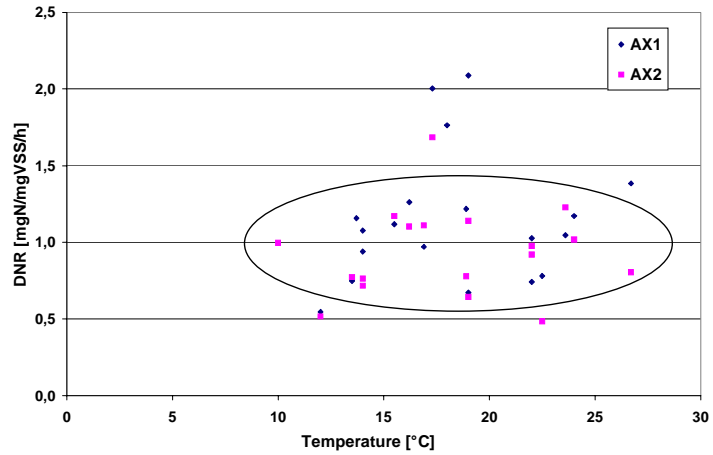
Outcomes

- Effective Bio-P in MBR from 8d up to 26d SRT
- 50µgP/L reached in > 80%
- 20µgP/L: refractory fraction
- MBR is advantageous for Bio-P process (complete retention + sludge conditioning before effluent extraction)
- But: natural precipitation in Ruhleben (Fe, Ca)

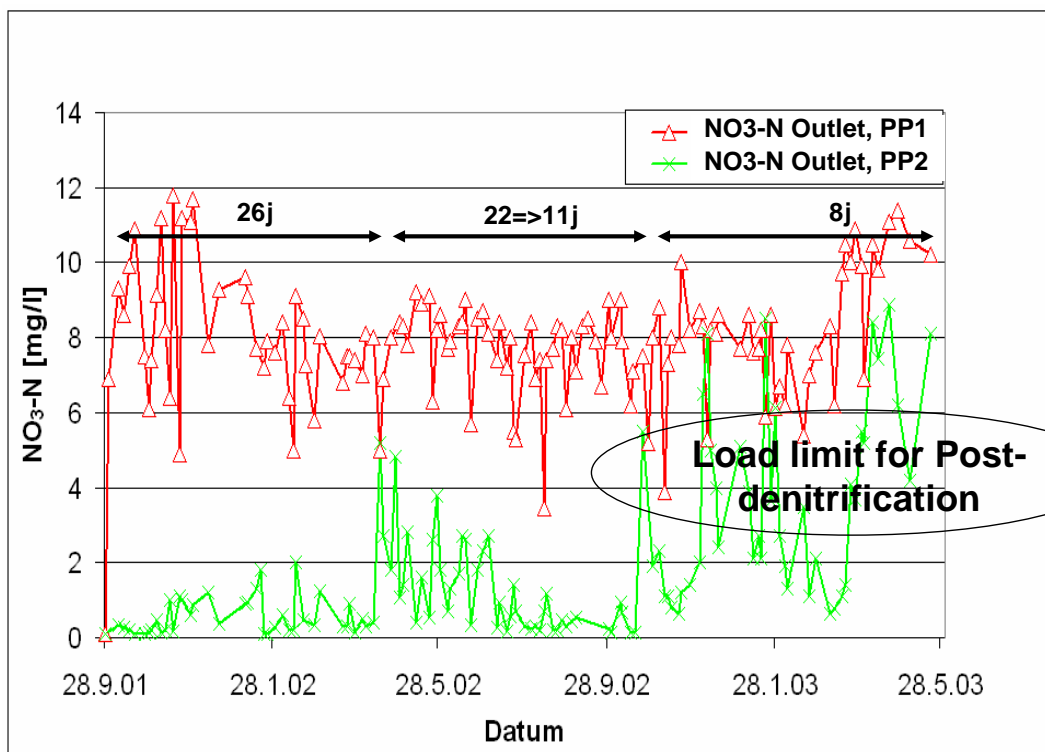


→ Layout with two anoxic reactors:

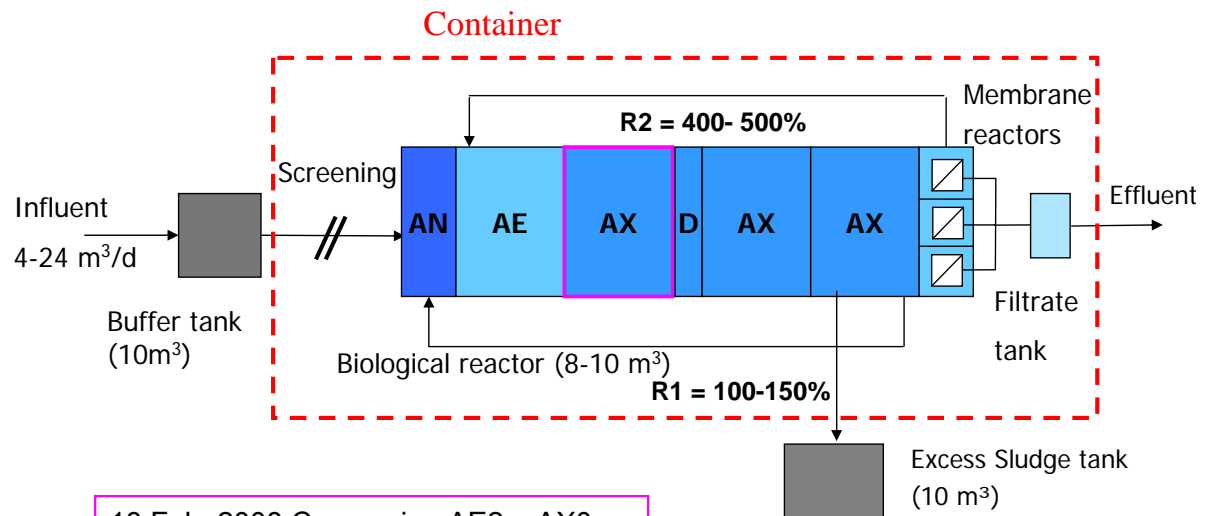
- DN-rate with 0.9 – 2.0 mgN/gVSS/h
- DN-Rate from Pilotplants proven
- Carbon source still unknown and under investigation
- Nitrate removal not always sufficient due to overload of the biological step



Anoxic zone



=7mg/L
Ruhleben
WWTP



18 Feb. 2008 Conversion AE2 – AX0

33. OPERATIONAL EXPERIENCES WITH THE HYBRID MBR HEENVLIET, A SMART WAY OF RETROFITTING

J.W. Mulder

Operational experiences with the hybrid MBR Heenvliet, a smart way of retrofitting

Jan Willem Mulder
Water Authority Hollandse Delta

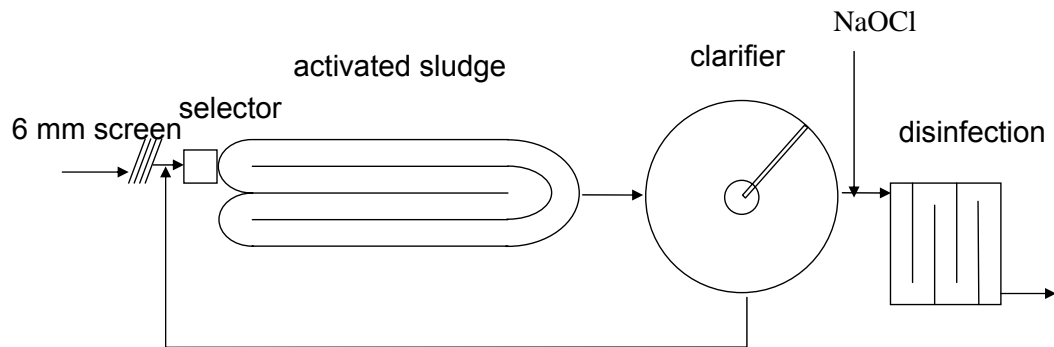
(Final MBR-Network workshop, Berlin, 31st March 2009)



Content

- ▶ wwtp Heenvliet, old situation
- ▶ Project objectives
- ▶ Upgrading of the wwtp
- ▶ Research
- ▶ Results
- ▶ Conclusions

wwtp Heenvliet, old situation

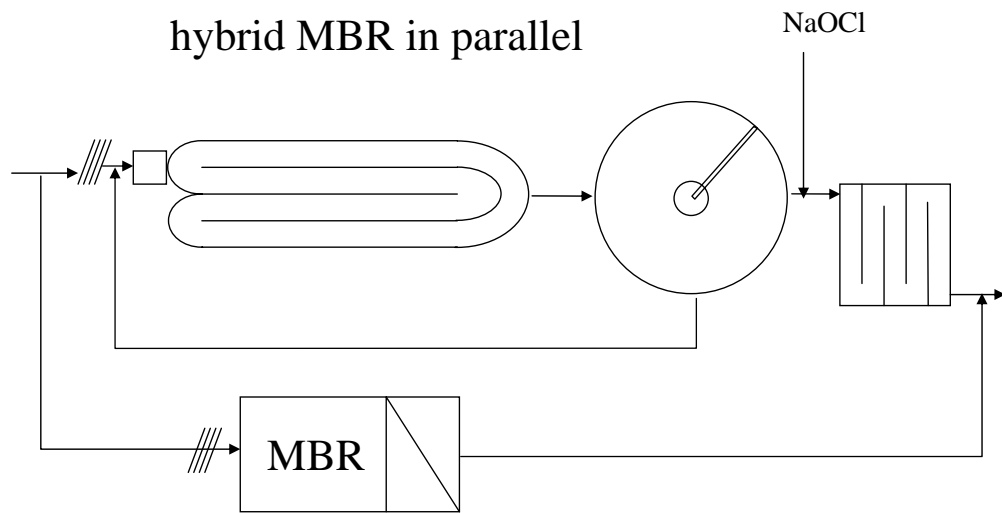


capacity: 9,000 p.e.
420 m³/h

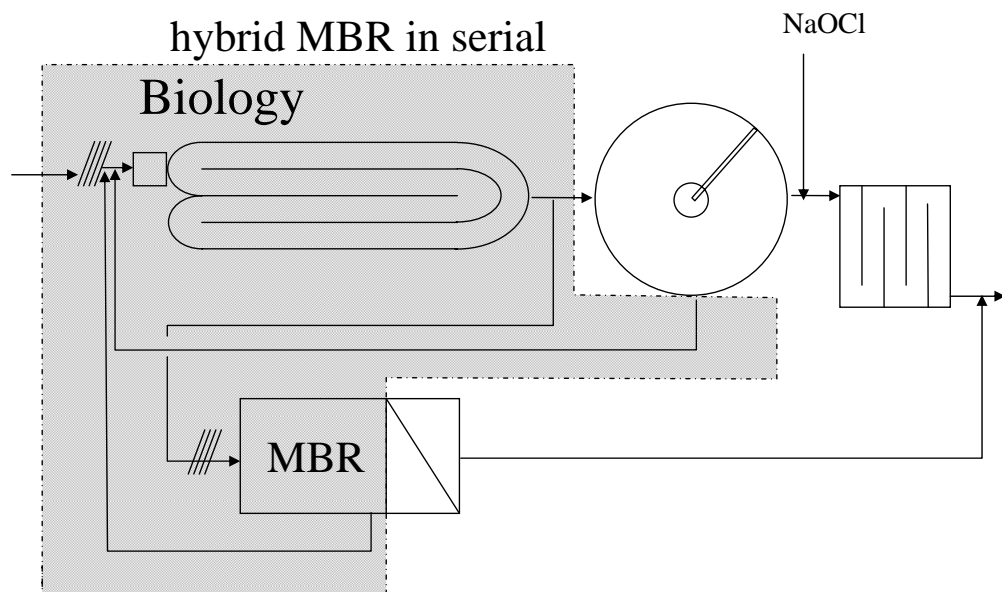
project objectives

- ▶ Upgrade wwtp for future growth of wastewater quantity
- ▶ Improvement effluent quality
target for N-total: 5 / 2.2 mg/l
target for P-total: 0.3 / 0.15 mg/l
E-coli: < 20/ml
- ▶ Contribution to MBR development for domestic wastewater treatment
- ▶ Own experiences with new technologies
- ▶ Hybrid MBR as model for upgrading of wwtp's

Options for wwtp upgrading 1



Options for wwtp upgrading 2



advantages option 2

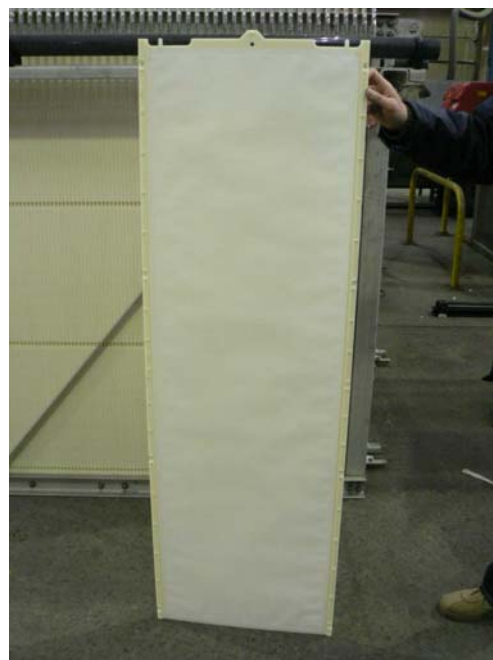
- ▶ Only one biological system
- ▶ flow splitting to membranes and clarifier independent from the biological performance
- ▶ More water treated by membranes; better use of the membranes
- ▶ More water treated by membranes; better mixed effluent quality

disadvantages option 2

- ▶ Higher average load to the membranes, what about membrane fouling?
- ▶ Sludge quality requirements for both membrane filtration and for sedimentation
- ▶ Complex process control

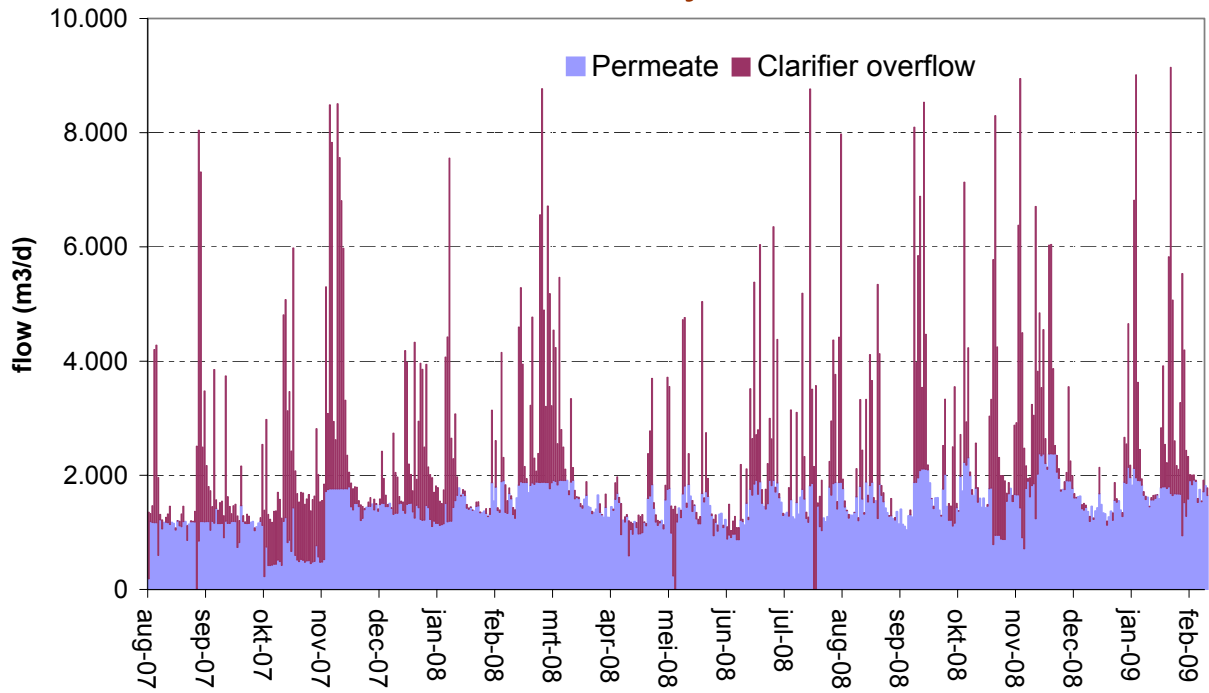
design hybrid MBR Heenvliet

- ▶ 9,000 pe → 13,000 pe
- ▶ 420 m³/h → 400 m³/h
- ▶ Max. flow to the membranes: 100 m³/h
- ▶ Toray flat sheet membranes
- ▶ material: PVDF; pore size 0.08 μm
- ▶ flux: 24.3 mlh
- ▶ 3,000 FS elements, divided over 16 modules in 2 membrane tanks
- ▶ max flow to clarifier: 300 m³/h; improvement of performance

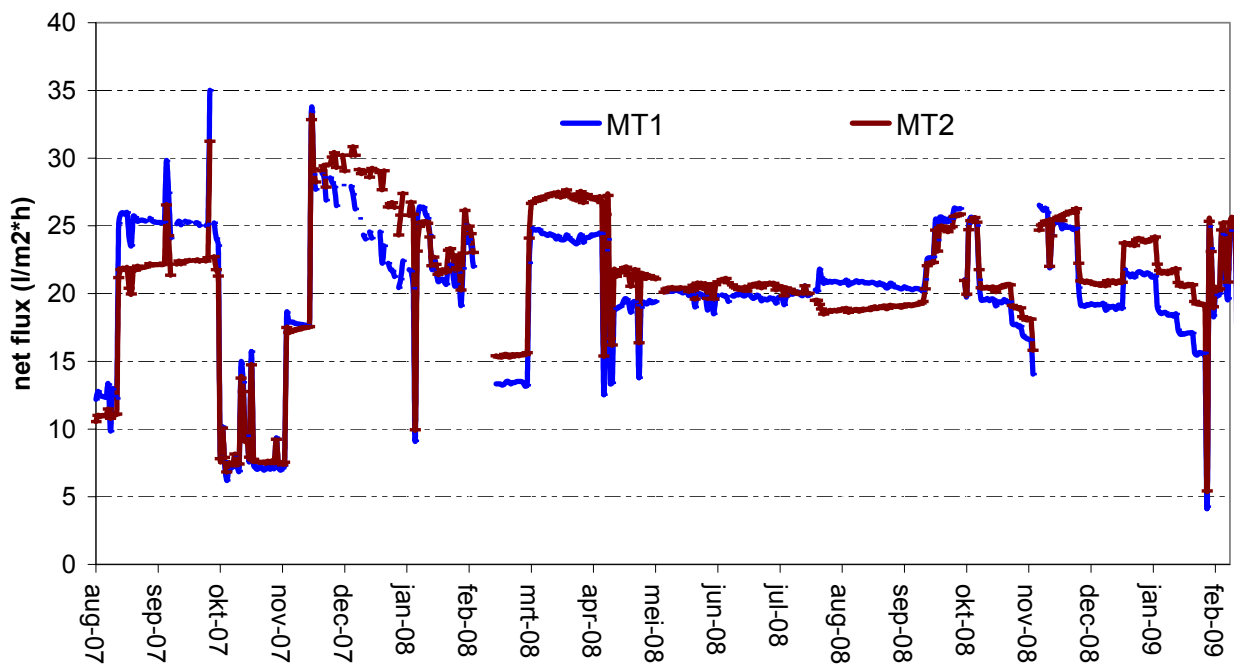




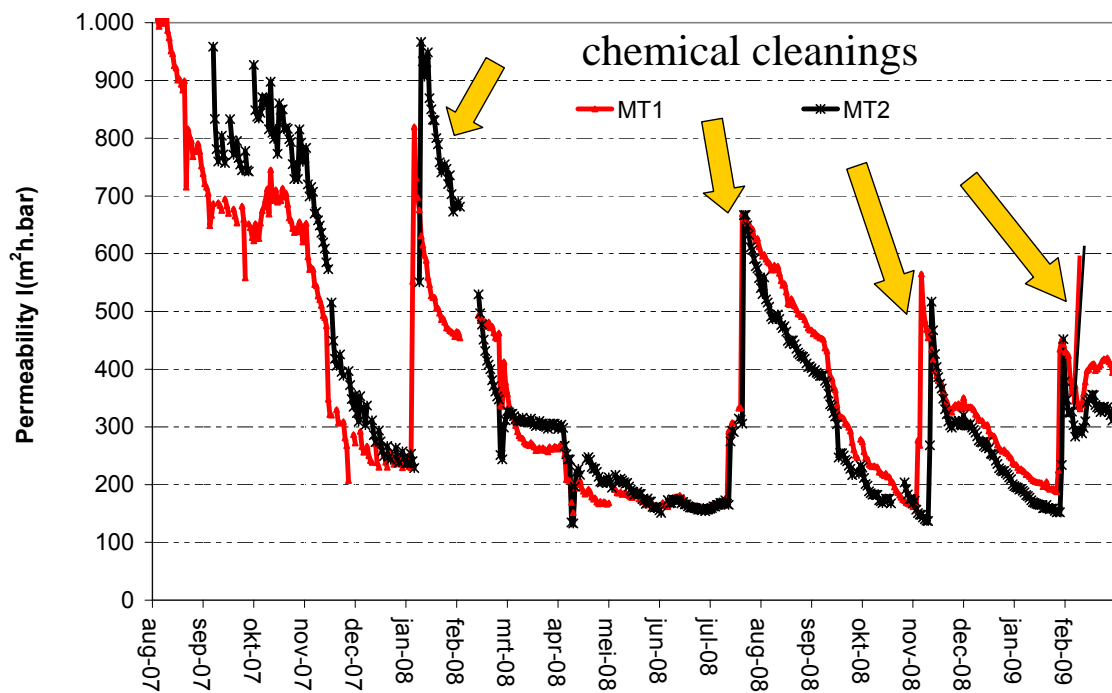
results hydraulic



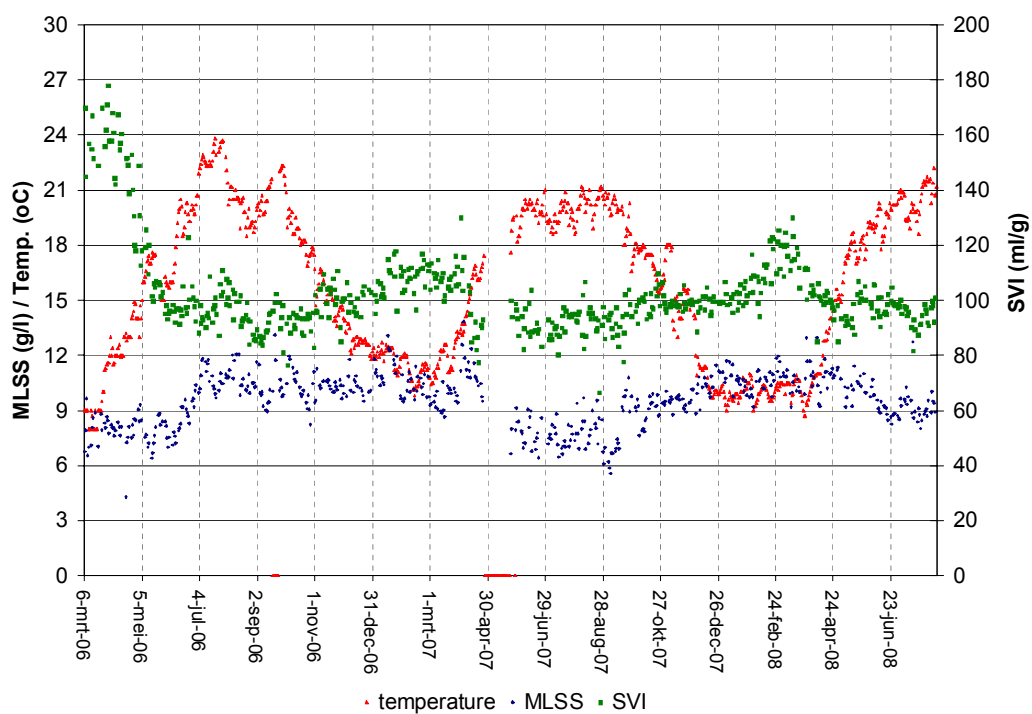
results, membrane performance



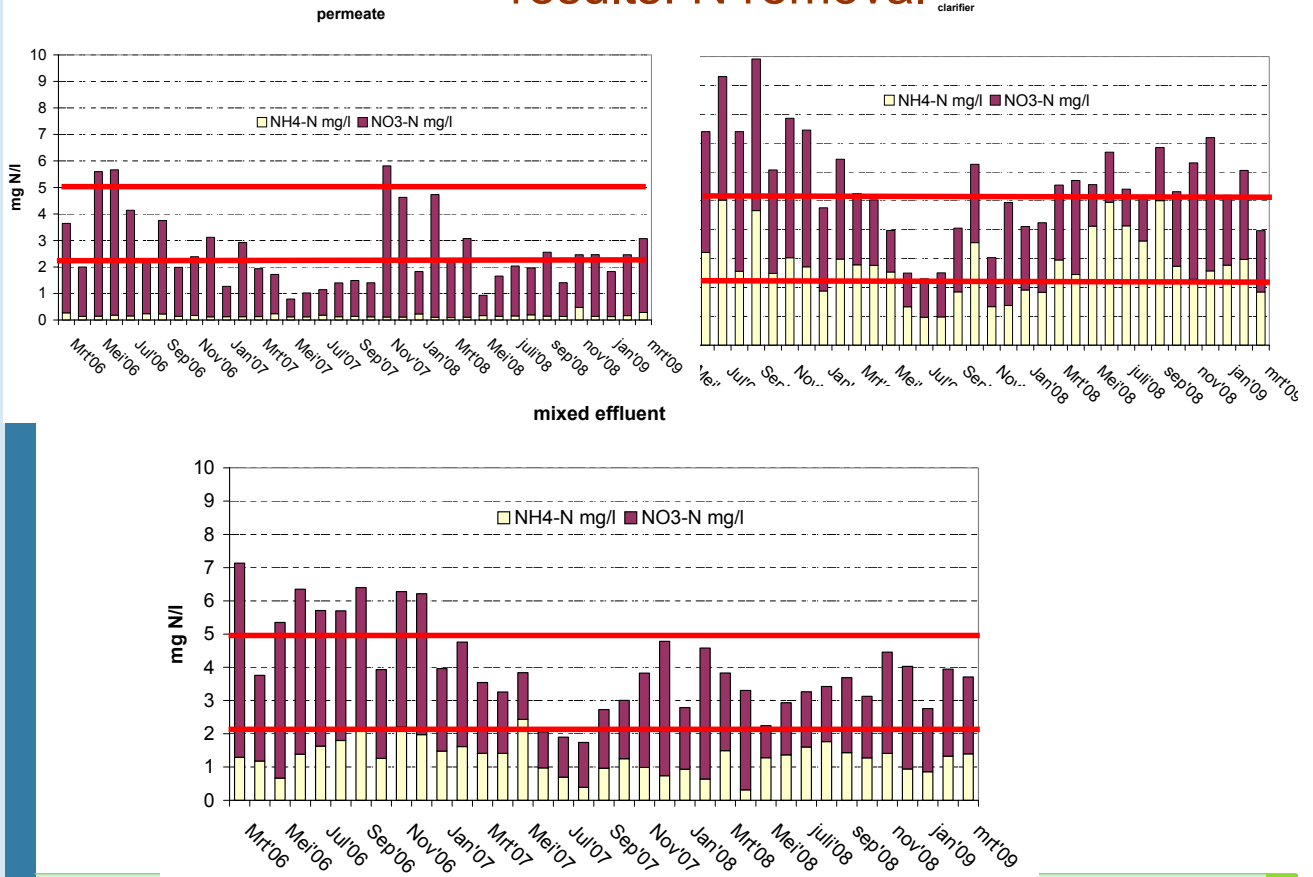
Results: permeability normalized to 15°C



Results: sludge quality



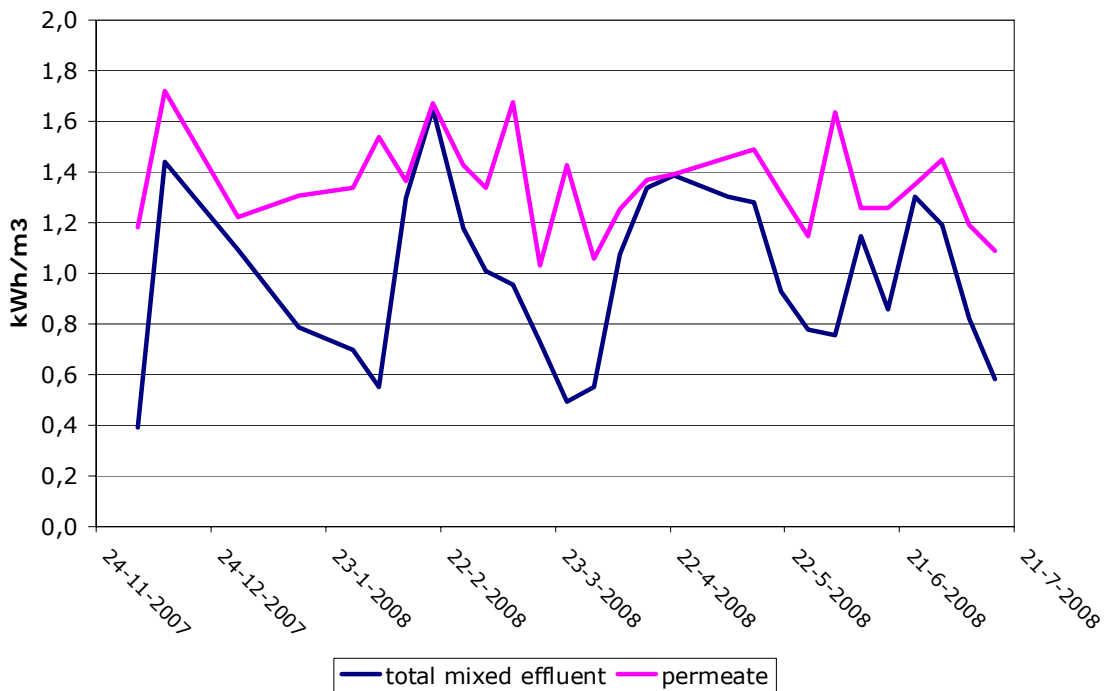
results: N removal



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Energy consumption



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Energy: possible optimizations

- ▶ Membrane aeration
 $0.45 \text{ Nm}^3/\text{m}^2\cdot\text{h} \rightarrow 0.3 \text{ Nm}^3/\text{m}^2\cdot\text{h} \rightarrow ??$
- ▶ On/off aeration in membrane tank
- ▶ Further optimizations in the biology
- ▶ Increasing the flux

Conclusions

- ▶ Good performance, but at relative low permeability
- ▶ Equilibrium in permeability dependant on performance
- ▶ Low frequent chemical cleaning
- ▶ The concept with one biology in two tanks need several optimizations
- ▶ Energy consumption need further optimizations

The success of the hybrid MBR

After the Heenvliet project 2 more hybrid MBR projects in the Netherlands

- ▶ wwtp Ootmarsum
by Water Authority 'Regge & Dinkel'
(2007)



- ▶ wwtp 'de Drie Ambachten', Terneuzen
Water Authority 'Zeeuws Vlaanderen' and Evides Water
Company (2009)



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Acknowledgement

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by the European Commission
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Duration: 01/10/05 – 31/05/09
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34. CHALLENGES AND POTENTIALS OF BIOFILM-MBR FOR MUNICIPAL WASTEWATER TREATMENT

T.O. Leiknes, J. Phattaranawik, I. Ivanovic

Challenges and potentials of biofilm-MBR for municipal wastewater treatment.

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Outline

1. BF-MBR concept
2. Treatment performance / capacities
3. Limitations – fouling
4. Development strategies of the BF-MBR concept
5. Illustration of three alternative system designs
6. Summary and conclusions

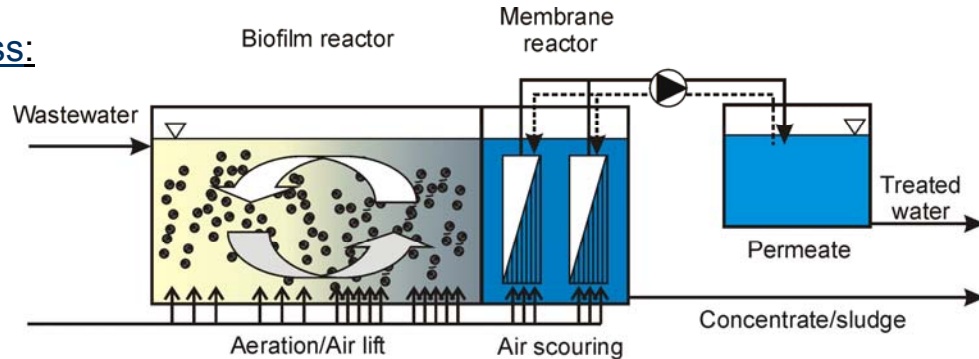
BF-MBR concept

Concept:

Biofilm process → Membrane process

SCOD → PCOD

Process:

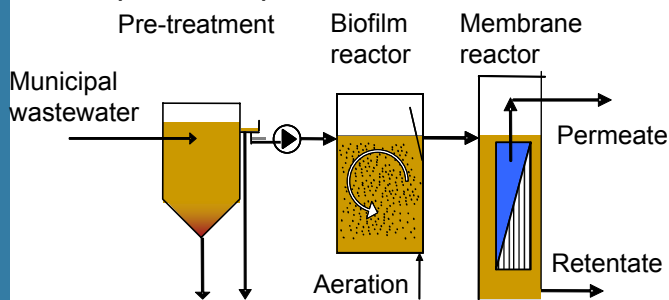


- One-step biological degradation – biological reactor configuration
- Design membrane reactor for enhanced particle removal
- Reduce MLSS concentration in influent to membrane reactor
- Reduce foulants in membrane reactor, i.e. colloidal fraction
- Enhanced performance, i.e. fouling control in membrane reactor

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BF-MBR applied to municipal wastewater

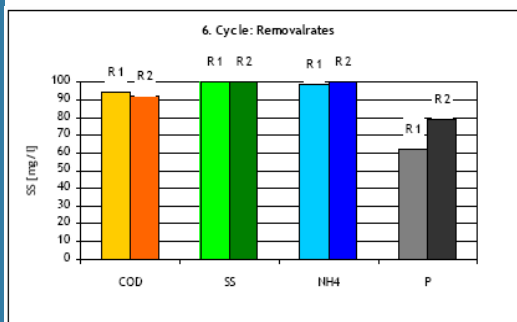
Pilot plant setup:



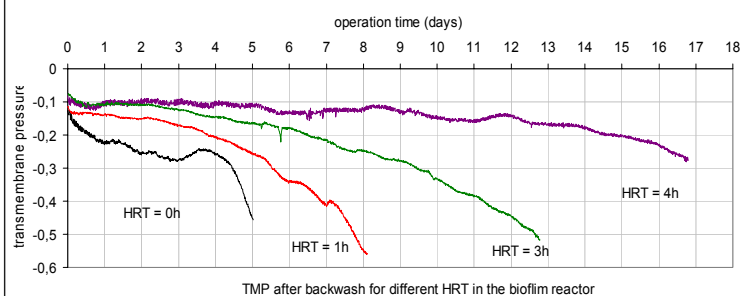
Operating conditions:

- varying organic loading rates
- varying suspended solids loads
- flux range: 35-60 LMH
- varying aeration intensities
- recoveries > 95%

Treatment efficiencies:

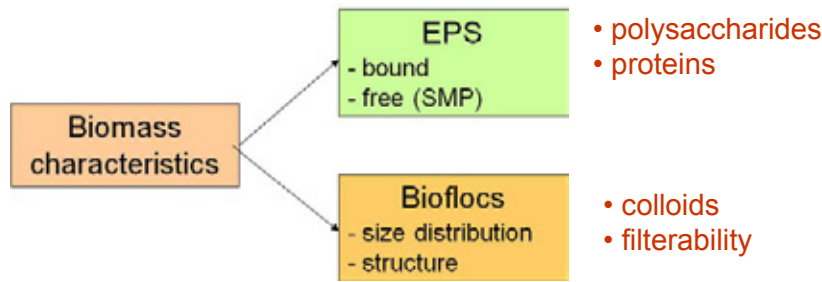


Membrane performance:



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Identification of dominant foulants in BF-MBR



- polysaccharides
- proteins
- what is significant?
- what dominates?

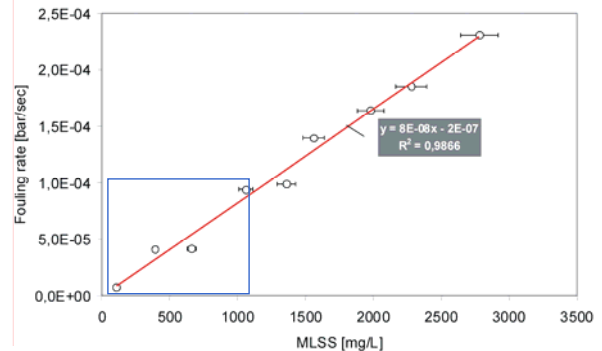
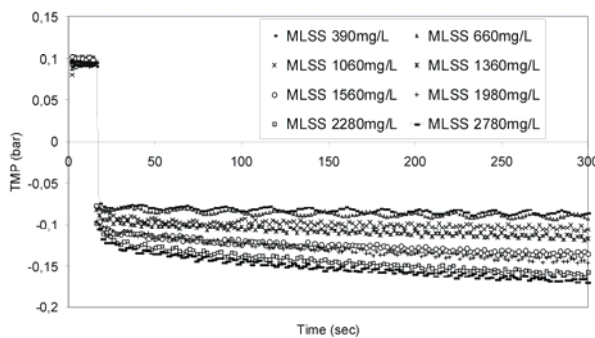
- colloids
- filterability



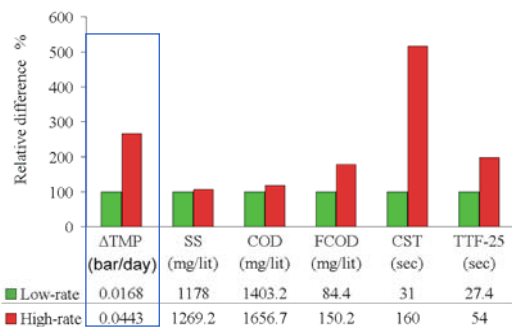
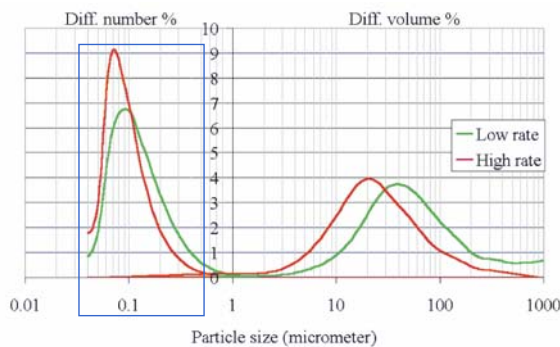
In BF-MBR results indicate that;

- suspended solids and particulates appear to dominate
- particularly the colloidal fraction is seen to impact performance

1. Suspended particles – MLSS - range > 1,2 µm



2. Colloidal particles – PSD number % - range 0,04 -1,2 µm



Advantages of BF-MBR over AS-MBR

Potentials of the BF-MBR:

- ▶ Very low suspended solid concentrations in BF-MBR
- ▶ Control of solid retention time (SRT)

Resulting in:

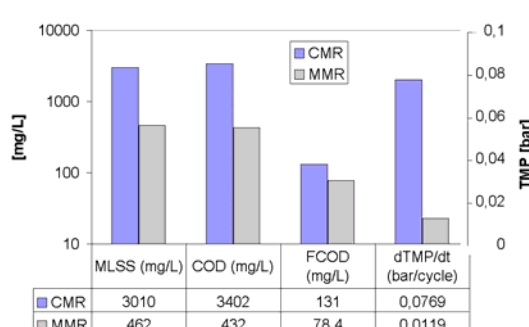
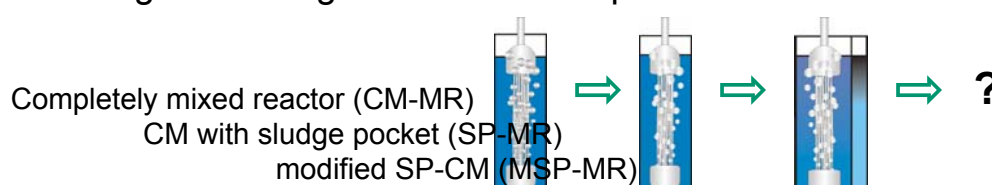
1. Enhanced fouling control by air scouring / hydrodynamics
2. Less problems with membrane clogging and plugging
3. Great flexibility for the process design: biological location and hydrodynamic arrangement etc.
4. Low viscosity: lower energy consumption for aeration
5. Higher membrane packing densities possible – more compact
6. Potentially less energy demanding

Development strategies for BF-MBR (1)



Development of membrane module / filtration unit:

- alternative filtration unit design / operation
- integrated designs for enhanced particle removal / solids control



Example of impact on unit design / operation

Development strategies for BF-MBR (2)



Development of integrated process configurations:

- suspended solids management – reactor design / operating modes
- “hybrid” processes – integrating alternative reactor options
- low TSS from bioreactor to membrane unit
- keep and remove excess sludge in the bioreactor

Three BF-MBR processes investigated within Eurombra

System I. Aerated moving-bed biofilm integrated with thickener design as double-deck biofilm reactor.

System II. Aerated moving-bed biofilm integrated with floating-bed anaerobic biofilm reactor.

System III. Aerated moving-bed biofilm integrated anaerobic biofilm with bubble-driven mixing device and integrated with thickener design.

System I - Double-deck aerobic biofilm MBR

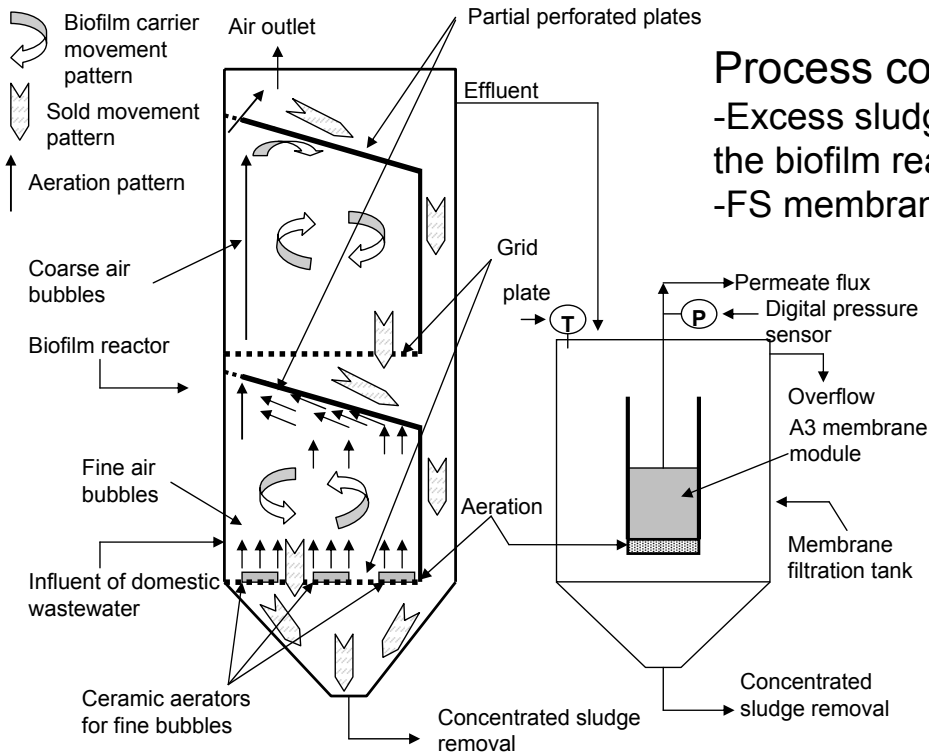
Objective:

Aerated moving-bed biofilm integrated with thickener design



Development goals:

- Reduction of membrane clogging problem potential
- Keep and remove excess sludge in biofilm reactor
- TSS in effluent from the bioreactor : 30-80 mg/L
- Reduce / control membrane fouling
- Investigate new FS module design



Process configuration

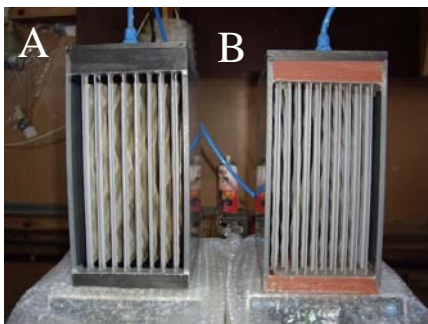
- Excess sludge removed from the biofilm reactor
- FS membrane module

Membrane module applied in investigation:

- FS membranes supplied by partner A3.
- Two modules designs compared and evaluated
 - standard module for AS-MBR
 - modified module for the BF-MBR
- Membrane filtration performance and clogging assessed

Potential benefit:

- Less clogging → increase membrane packing density
- Improved hydrodynamics → less energy for air scouring

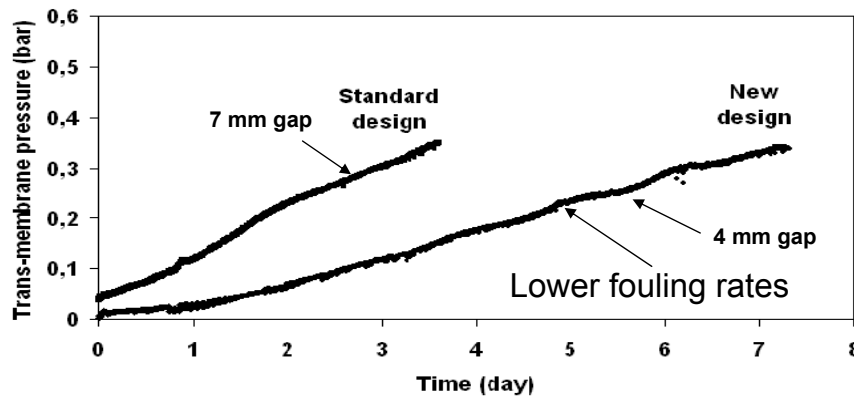


A: A3 module for AS-MBR: 7 mm gaps

B: A3 module for BF-MBR: 4 mm gaps

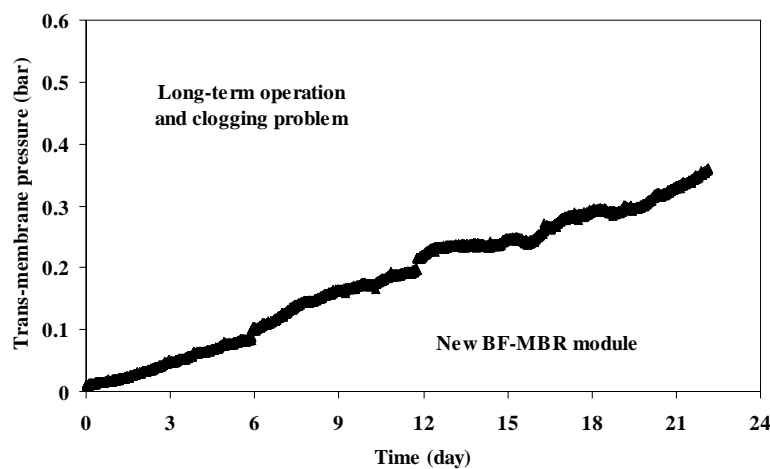
System I – results:

Comparison of the two modules:



- operating conditions; flux 18 L/m²·h, aeration of 32 L/min
- average TSS; in influent – 44.6 mg/l, membrane tank 100-140 mg/L
- fouling (dTMP/dt); 0.086 bar/day, 0.046 bar/day new design
- superficial air velocity increased by 50%
- modified module occupied 35% less space

Membrane clogging test for long-term operation:

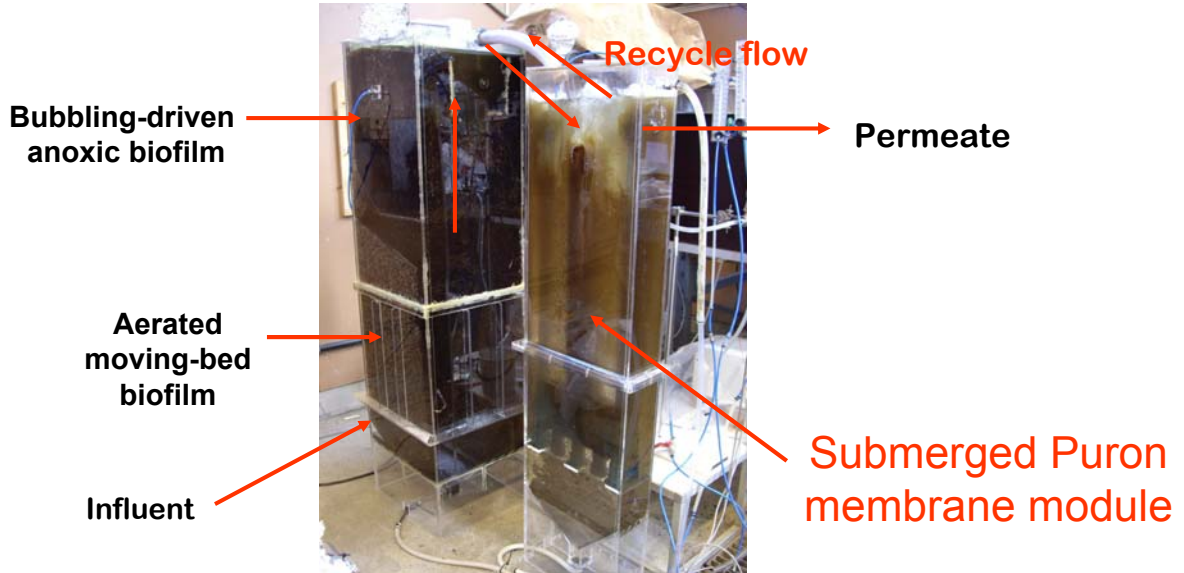


- Linear TMP development found
- No membrane clogging in the new 4 mm gap module

System II - Aerobic-anoxic biofilm MBR with energy recovery unit

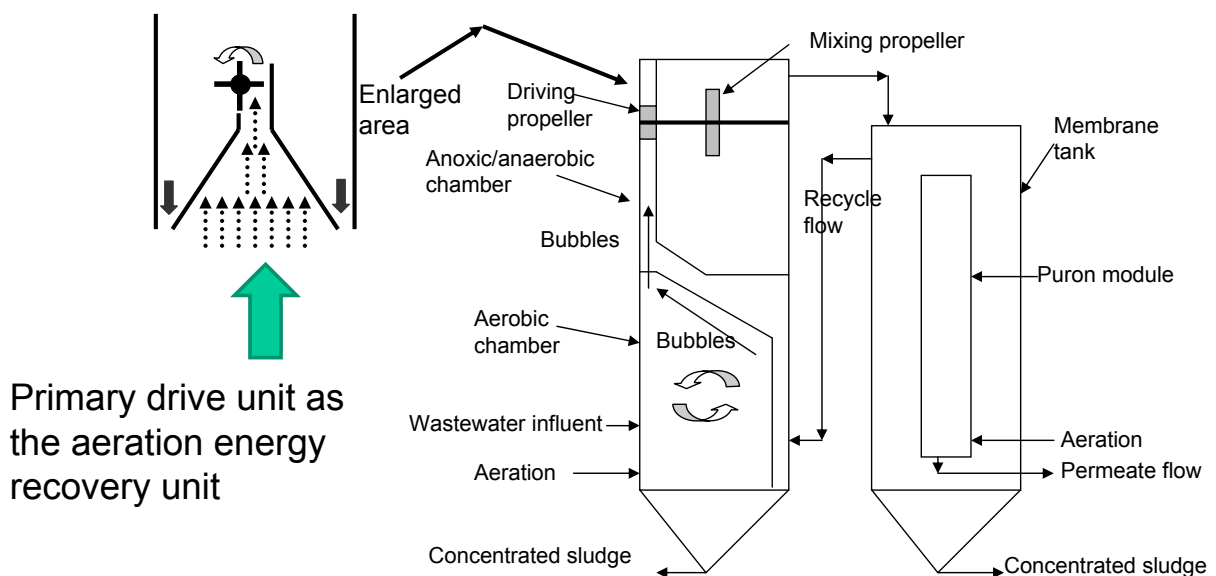
Objective:

Integrated thickener design, bubble-driven mixer in anoxic zone



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Process configuration for System II



Excess sludge removed from the biofilm reactor and membrane tank to control TSS

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Membrane module applied in investigation:
-Membrane module supplied by partner KMS, Puron.

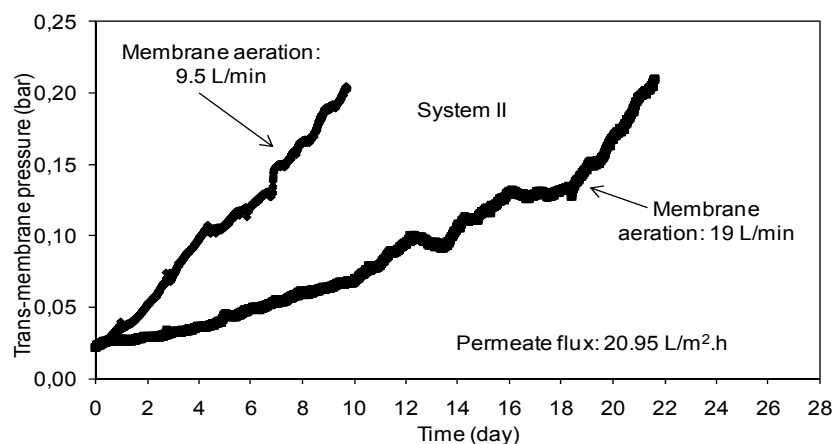
-Membrane filtration performance and clogging assessed



- TSS in effluent from the bioreactor: 20-50 mg/L.
- Membrane clogging problem was not found for long-term test.
- The removed concentrated sludge for sludge disposal has concentration of ~ 15 g/L.
- The rotating speed of mixer ~ 55 rpm.
- Operating conditions; flux 21 L/m²·h, aeration of 9.5 and 19 L/min

System II - results

Effect of membrane aeration intensity:

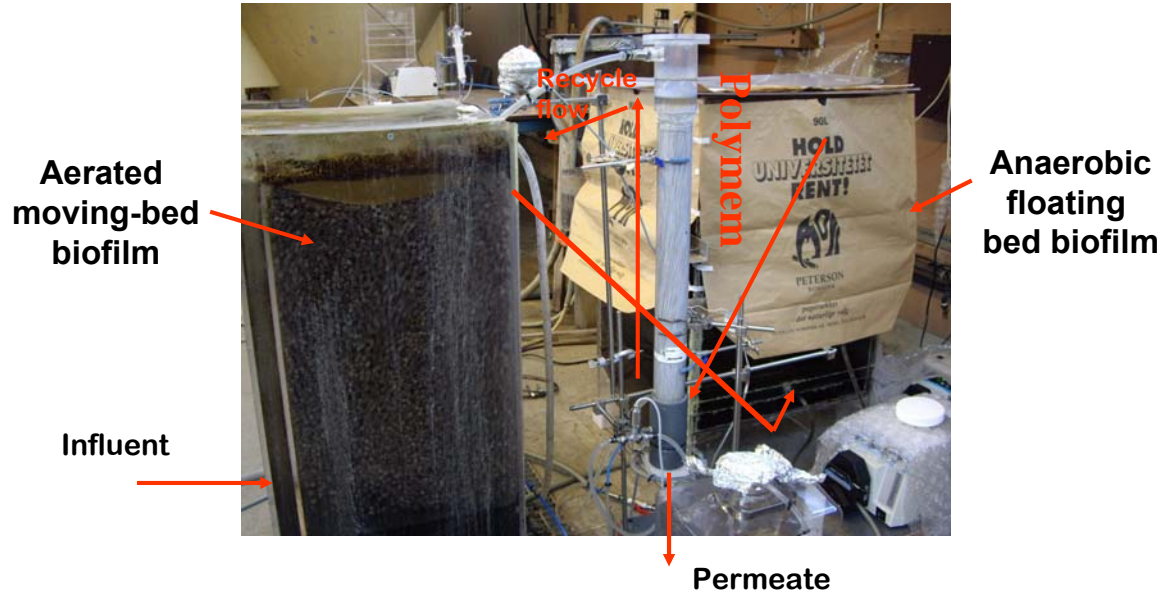


Average fouling rates

- 0.019 bar/day at aeration rate 9.5 L/min (2 cm/s superficial air velocity)
- 0.0091 bar/day at aeration rate 19 L/min (4 cm/s superficial air velocity)

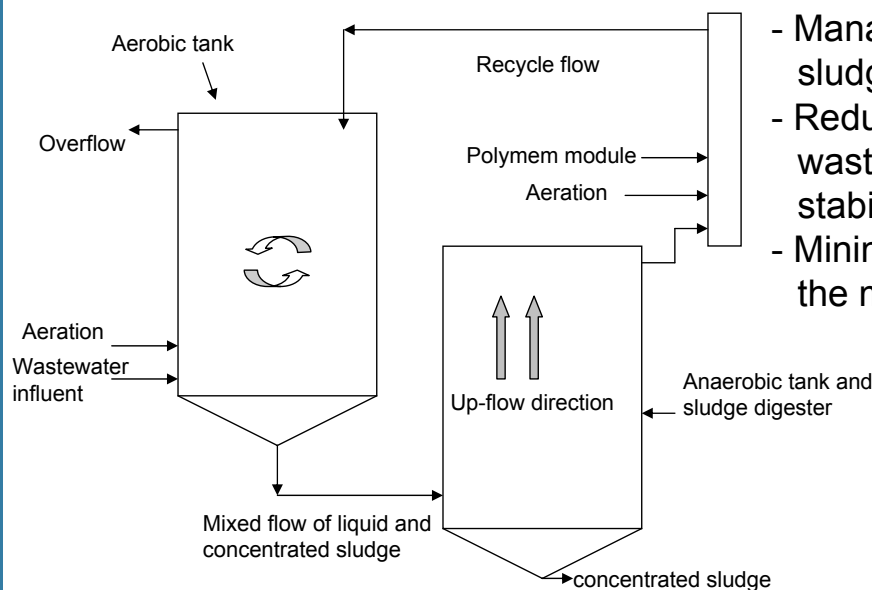
System III - Aerated moving-bed biofilm with floating-bed anaerobic biofilm

Objective:
Sludge management, anaerobic sludge stabilization



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Process configuration for System III



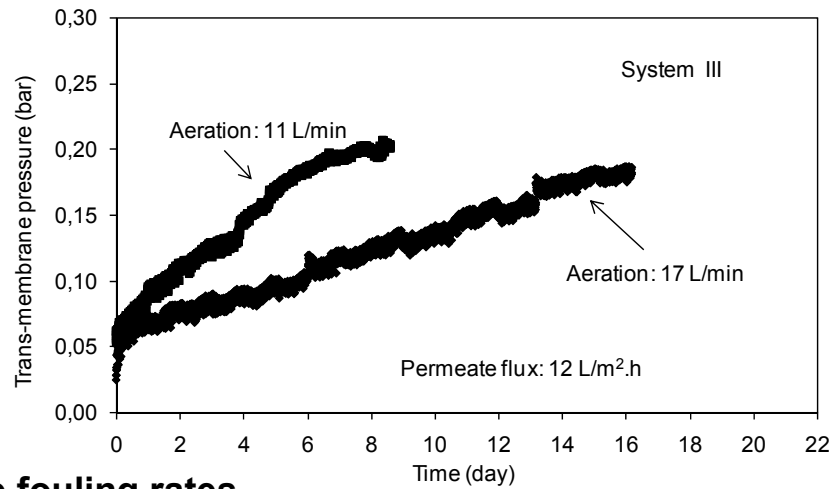
- Management of the excess sludge
- Reduction of excess sludge waste by anaerobic sludge stabilization concept.
- Minimize TSS in the flow to the membranes:

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System III - results

Membrane module applied in investigation supplied by partner Polymem

Effect of membrane aeration intensity:



Average fouling rates

- 0.02 bar/day at aeration rate 11 L/min (5 cm/s superficial air velocity)
- 0.012 bar/day at aeration rate 17 L/min (8 cm/s superficial air velocity)

Conclusions

Potentials of BF-MBR

- ▶ New and flexible process configurations possible
 - Alternative strategy for solids control and management
 - Lower suspended solids load on membrane
 - Minimal clogging/sludging problems
 - Enhanced membrane performance / less fouling
 - Lower energy requirements (overall)

Challenges of BF-MBR

- ▶ Understanding membrane fouling
- ▶ Process control and optimization
- ▶ System complexity and interdependence

Acknowledgement

EUROMBRA is a research project supported by the European Commission under the Sixth Framework Programme (Priority “Global Change and Ecosystems”)



Contract No. 018480 - EUROMBRA
Duration: 01/10/05 - 31/05/09
EUROMBRA is part of the MBR-NETWORK Cluster



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35. ANAEROBIC MEMBRANE BIOREACTOR FOR DOMESTIC WASTEWATER TREATMENT

*G. Gugliemi, D. Avesani, C. Cabassud, V. Yvenat,
S. Pollet, O. Lorain, A. Schmidt, B. Pfaff, S. Pillay,
K. Foxon, J. K. Bwapwa, C. Buckley*

MBR-Network
Final workshop
31st March – 1st April 2009
Berlin

Anaerobic Baffled Reactor - Membrane Combination

Sudhir Pillay, Joseph Bwapwa
, Kitty Foxon, Chris Brouckaert, **Chris Buckley**, *Diego Avesani, *Giuseppe Guglielmi, **Corinne Cabassud, Valentin Yvenat

University of KwaZulu-Natal, South Africa

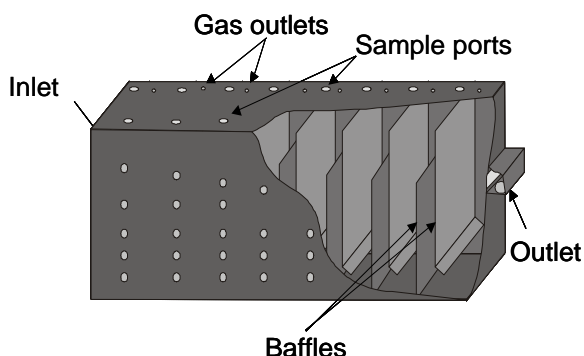
*Università degli Studi di Trento, Italy

**INSA - Toulouse

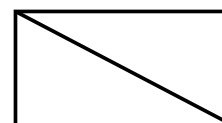


Aims

- ▶ Combine anaerobic baffled reactor with membranes
 - developing country context
 - no external energy, quiescent flow conditions
 - low per capita water consumption (50 L/d free basic water)
 - pour – flush toilets (using grey water)
 - decentralised system
 - reuse of pathogen-free water for urban agriculture
 - low biomass production and membrane operating conditions



+



Conditions

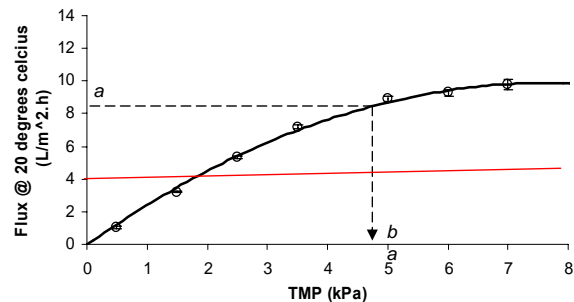
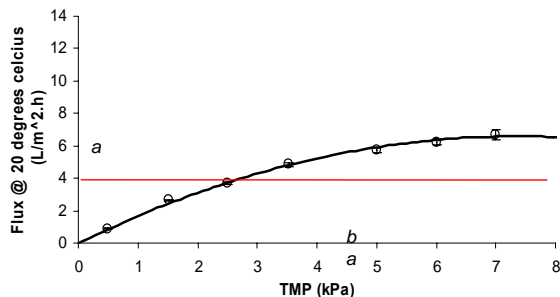
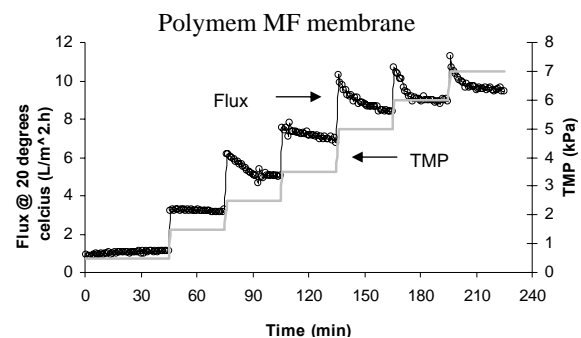
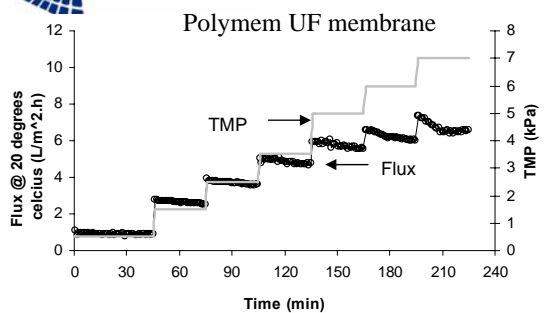
feed: solution made from pit latrine sludge

ABR: feed tank (200 L) + 4 baffled reactors (20 L each)

HRT: ~ 3 days

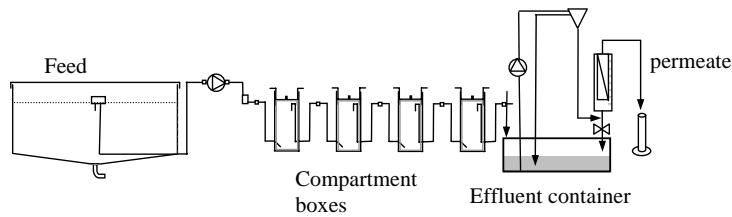
membrane: Polymem MF and UF modules

Membrane	Type	Area	Pore size	Model	Supplier
Ultrafiltration	Hollow-fibre	1 m ²	0.08 µm	UF2522 S3	Polymem, Fr.
Microfiltration	Hollow-fibre	1 m ²	0.20 µm	MF2522 S	Polymem, Fr.



Parameter	Raw Feed	Effluent		MF		UF	
	COD (mg/L)	COD (mg/L)	Removal (%)	COD (mg/L)	Removal (%)	COD (mg/L)	Removal (%)
COD _(total)	1 304 ± 101	234 ± 4	82	34 ± 8	86	18 ± 4	92
COD _(0.45 µm)	391 ± 35	95 ± 14	76				

ABR-MBR Cleaning



ABR					
	Operation (d)	HRT (d)	Feed COD (mg/L)	Outlet COD (mg/L)	
Run 1	267	3	687	252	★
Run 2	124	3	1 561	303	
Run 3	32	2	1 036	310	★
Run 4	28	2	1 500	334	★
Run 5	20	3	2 000	457	
Run 6	12	3	3 000	457	★

★ TMP-Step experiments

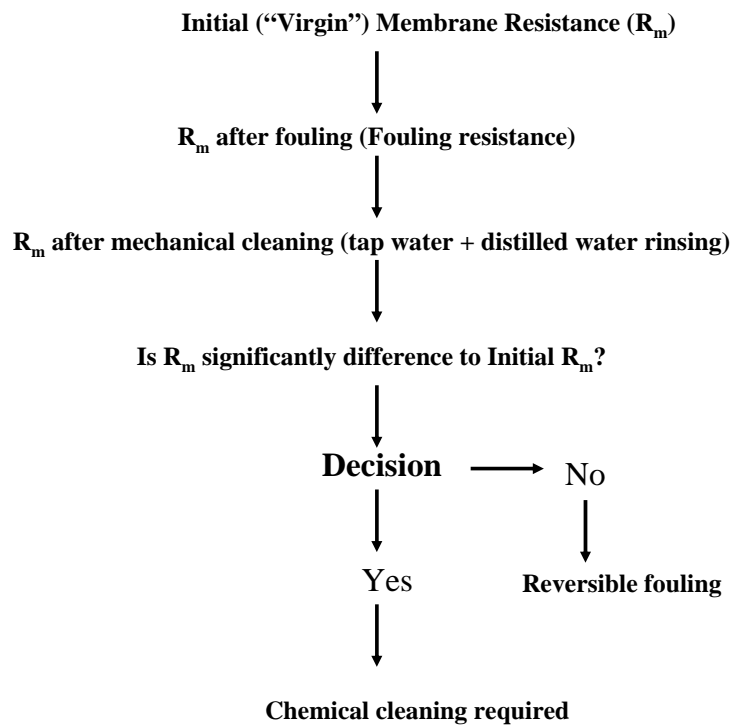
Diapositive 5

92

Activated sludge tests not very replicable..as u can be by stdev curves...probably membrane near end of its life. If u look at other tests where there was less membrane usage (AD sludge for example, 3 tests, achieve quite gud results, same with ABR effluent [C4 overflow if anyone asks]).

991239853; 05/12/2007

Membrane Cleaning

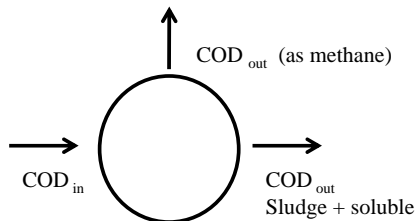


Membrane Resistance

	Stage of Filtration	MF ($\times 10^{12} \text{ m}^{-1}$)	UF ($\times 10^{12} \text{ m}^{-1}$)
Run 1	Initial ‘Virgin’ Membrane Resistance	3.62	3.62
	Pure water flux after TMP-step	3.65	3.64
	Mechanical Cleaning		
	Pure water flux before 4h filtration at 4 kPa	3.64	3.64
	Pure water flux after 4 h filtration at 4 kPa	3.66	3.67
	Mechanical Cleaning		
Run 3	Pure water flux after TMP-step	3.63	3.62
	Pure water flux after TMP-step	3.69	3.65
	Mechanical Cleaning + Chemical Cleaning (2000 ppm Chlorine for 1 h)		
Run 4	Pure water flux after TMP-step	3.60	3.60
	Pure water flux after TMP-step	3.61	3.60
	Mechanical Cleaning		
Run 6	Pure water flux after TMP-step	3.60	3.60
	Pure water flux after TMP-step	3.61	3.62

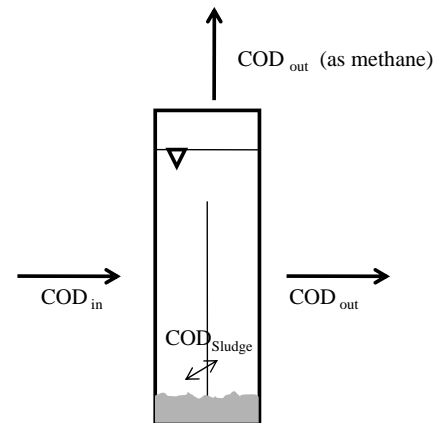
Anaerobic Sludge Production (1 of 3)

CSTR anaerobic digester



sludge production leaves the reactor with the effluent

The ABR does not operate in a steady-state mode. It operates as a flow-through accumulation system



most of the produced sludge stays in the anaerobic compartments

Anaerobic Sludge Production (2 of 3)

The model is based on a steady-state modeling

- Kinetic part → predicts % COD removal and methane production
- Stoichiometry part → predicts the gas composition, **effluent free and saline ammonia**

a factor f is introduced which represents the fraction of S_{bp} and Z_{ad} that leave the reactor

From the model we know the biodegradable particulate COD and Acidogen biomass in the effluent:

From the experimental data we know the Total COD in the effluent:

This means that we can know the unbiodegradable particulate in the sludge

Anaerobic Sludge Production (3 of 3)

		Average/ Median	Std Deviation	Number of observations	Min.	Max.
COD	in	688	204	202	246	1749
	[mg COD/ℓ] out	130	64	18	62	339
Alkalinity	in	256	38	190	142	369
	[mgCaCO ₃ /ℓ] out	246	53	4	168	286
Ammonia	in	40	11	189	11	95
	[mgN/ℓ] out	51	23	10	20	90
VFA	in	33	34	4	0	79
	[mgCH ₃ COOH/ℓ] out	nd				
pH	in	7.2		195		7.8
	[Median value] out	6.5		6	6.2	7.4

Sludge accumulation rate [kg dry solids/year]	
Model	Experimental data
52.74	60.70

The model seems able to predict the sludge despite the low numbers of experimental data and their high standard deviation

Conclusions

- ▶ anaerobic biological system
- ▶ low TS in membrane compartment (10 to 50 mg/L)
 - anaerobic baffle reactor is a solids accumulating system
- ▶ long term operation at 400 mm water head
- ▶ simple water clean
- ▶ spend resources on membrane area not energy

Next Steps (mid 2009)

- ▶ municipality + Borda are installing a **technical evaluation plant**
 - 84 houses
 - at a permaculture site
 - agricultural reuse (water + N + P)
 - evaluation of long-term flux + operating and maintenance requirements
- ▶ town planning objective
 - compact low-income housing

Acknowledgement

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**South African
Water Research Commission**

